

Product Submittal/Substitution Request

T0:				
PROJECT:				
PROJECT LOCATION	:			
SPECIFIED ITEM:				
Section	Page	Paragraph	Description	
PRODUCT SUE	BMIT TAL / SUBST	ITUTION REQUESTED:		
The attached submittal SUBMITTED B		uct description, specifications, drawing	is, and performance data for use in the evaluation of	of the request.
Name:			Signature:	
Company:			<u> </u>	
Address:				
Date:	Tele	ephone:	Fax:	
FOR USE BY 1	THE ARCHITECT A	ND/OR ENGINEER		
Approved	Approved as No	nted Not Approved		
(If not approved, pleas	se briefly explain why the p	roduct was not accepted.)		
Ву:			Date:	

Questions or inquiries? Contact us at engineering@powers.com, or call 1.800.524.3244





DEWALT® Screw-Bolt+™ Submittal Section:

Competitive Comparisons:

- DEWALT® Screw-Bolt+ vs. Hilti KH-EZ vs. Simpson Strong-Tie Titen HD
- DEWALT® Screw-Bolt+ vs. Hilti KH-EZ
- DEWALT® Screw-Bolt+ vs. Simpson Strong-Tie Titen HD

Product Pages:

- General Information
- Installation Instructions
- Design Tables
- Ordering Information



 $Off line\ version\ available\ for\ download\ at\ \underline{www.dewaltdesignassist.com}.$

DEWALT developed the DEWALT Design Assist (DDA) anchor software to enable users to input technical data into a dynamic model environment-to visualize, consider, and specify anchors in today's changing engineering climate.

For a demonstration of the latest version of PDA, contact us at anchors@DEWALT.com



COMPETITIVE COMPARISON

ANCHORS & FASTENERS

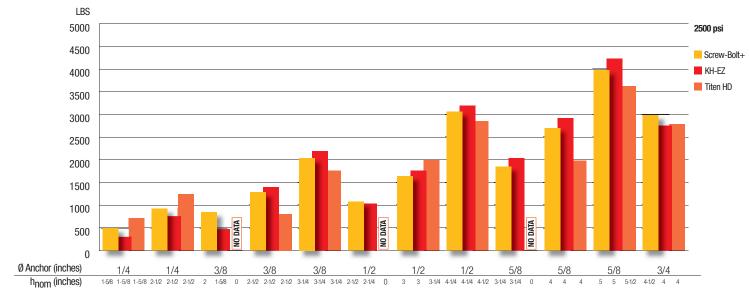
SCREWBOLT+™ VS. HILTI* KH-EZ VS. SIMPSON STRONG-TIE* TITEN HD

PRODUCT COMPARISON

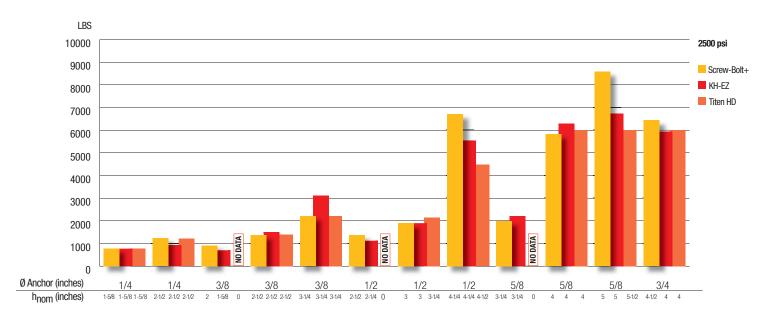
Product Name	Screw-Bolt+	KH-EZ	Titen HD
Company	DeWALT	Hilti*	Simpson Strong-Tie*
Description	Carbon Steel Screw Anchor	Carbon Steel Screw Anchor	Carbon Steel Screw Anchor
Size Range (inch)	1/4, 3/8, 1/2, 5/8, 3/4	1/4, 3/8, 1/2, 5/8, 3/4	1/4, 3/8, 1/2, 5/8, 3/4
ICC-ES ESR (concrete)	ESR-3889	ESR-3027	ESR-2713
Revision Date	2022 November	2021 December	2022 September
Cracked Concrete	Yes	Yes	Yes
Seismic	Yes	Yes	Yes
Concrete-filled Steel Deck	Yes	Yes	Yes
* Hilti is a registered trademark of Hilti Corporation	* Simpson Strong-Tie are registered trademarks of Simpson Strong-Tie	e Company Inc.	

PRODUCT PERFORMANCE COMPARISON

FACTORED TENSION LOADS CRACKED CONCRETE



FACTORED SHEAR LOADS CRACKED CONCRETE





COMPETITIVE COMPARISON

ANCHORS & FASTENERS

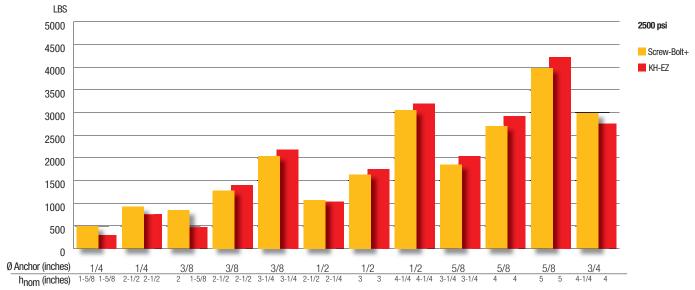
SCREWBOLT+™ VS. HILTI* KH-EZ

PRODUCT COMPARISON

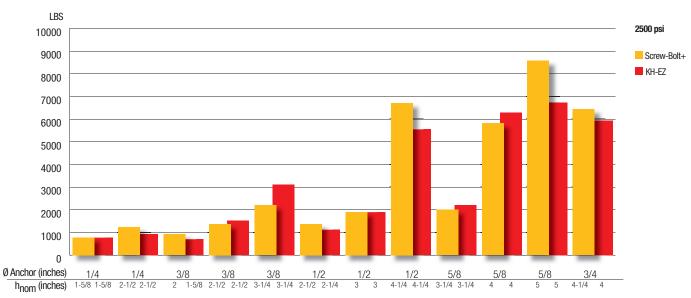
Product Name	Screw-Bolt+	KH-EZ
Company	DeWALT	Hilti*
Description	Carbon Steel Screw Anchor	Carbon Steel Screw Anchor
Size Range (inch)	1/4, 3/8, 1/2, 5/8, 3/4	1/4, 3/8, 1/2, 5/8, 3/4
ICC-ES ESR (concrete)	ESR-3889	ESR-3027
Issued	2022 November	2021 December
Cracked Concrete	Yes	Yes
Seismic	Yes	Yes
Concrete-filled Steel Deck	Yes	Yes
* Hilti is a registered trademark of Hilti Corporation		

PRODUCT PERFORMANCE COMPARISON

FACTORED TENSION LOADS CRACKED CONCRETE



FACTORED SHEAR LOADS CRACKED CONCRETE





COMPETITIVE COMPARISON

ANCHORS & FASTENERS

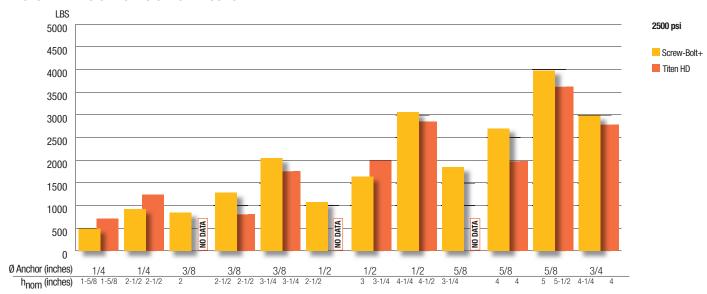
SCREWBOLT+™ VS. SIMPSON STRONG-TIE* TITEN HD

PRODUCT COMPARISON

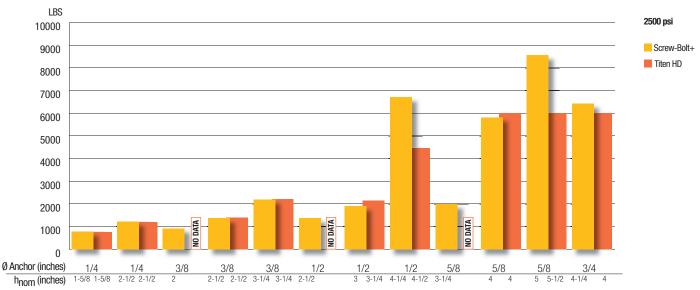
Product Name	Screw-Bolt+	Titen HD
Company	DeWALT	Simpson Strong-Tie*
Description	Carbon Steel Screw Anchor	Carbon Steel Screw Anchor
Size Range (inch)	1/4, 3/8, 1/2, 5/8, 3/4	1/4, 3/8, 1/2, 5/8, 3/4
ICC-ES ESR (concrete)	ESR-3889	ESR-2713
Issued	2022 November	2022 September
Cracked Concrete	Yes	Yes
Seismic	Yes	Yes
Concrete-filled Steel Deck	Yes	Yes
* Simpson Strong-Tie are registered trademarks of Simp	son Strong-Tie Company Inc.	

PRODUCT PERFORMANCE COMPARISON

FACTORED TENSION LOADS CRACKED CONCRETE



FACTORED SHEAR LOADS CRACKED CONCRETE





GENERAL INFORMATION

SCREW-BOLT+™

High Performance Screw Anchor

PRODUCT DESCRIPTION

The Screw-Bolt+ anchor is a one piece, heavy duty screw anchor with a finished hex head or flat head (countersunk). Suitable base materials include normal-weight concrete, sand-lightweight concrete, concrete over steel deck, concrete masonry and solid clay brick. It is simple to install, easy to identify and fully removable. The patented thread design, designed for use with standard ANSI drill bits, reduces installation torque and enhances productivity. The steel threads along the anchor body tap into the hole during installation to provide keyed engagement and allow for reduced edge and spacing distances. The Screw-Bolt+ is available with a standard bright zinc-plated finish or mechanically galvanized plating which offers more corrosion resistance.

- Racking, shelving and material handling
- Support ledgers and sill plate attachments
- · Barriers, guards and temporary supports
- Glazing and window attachments
- Retrofits, repairs and maintenance
- Fencing, railing and stair stringers
- · Cracked and uncracked concrete
- Wind and seismic loading (SDC A F)

FEATURES AND BENEFITS

- + Designed for use with ANSI standard tolerance drill bits
- + Patented thread design offers toughened threads for tapping high strength concrete
- + Low installation torque in concrete and masonry base materials
- + Universal product for concrete and grouted/solid masonry
- + Fast installation with powered impact wrench, but can also be installed manually
- + Can be installed closer to a free edge than traditional expansion anchors
- + Fully removable and reinstallable in same hole (see www.DEWALT.com)
- + Ratchet teeth on underside of hex washer head lock against fixture
- + Diameter, length and identifying marking stamped on head of each anchor

- International Code Council, Evaluation Service (ICC-ES), ESR-3889 for concrete
- International Code Council, Evaluation Service (ICC-ES), ESR-4042 for masonry
- Code Compliant with the International Building Code/International Residential Code: 2021 IBC/İRC, 2018 IBC/IRC, 2015 IBC/IRC, and 2012 IBC/IRC
- Tested in accordance with ACI 355.2/ASTM E488 and ICC-ES AC193 for use in concrete under the design provisions of ACI 318 (-19 and -14) Chapter 17 or ACI 318-11 Appendix D
- Evaluated and qualified by an accredited independent testing laboratory for recognition in cracked and uncracked concrete including seismic and wind loading (anchor Category 1 for all sizes)
- Evaluated and qualified by an accredited independent testing laboratory for sensitivity and reliability against brittle failure, e.g. hydrogen embrittlement
- City of Los Angeles, LABC Supplement (within ESR-3889 and ESR-4042)
- Florida Building Code, FBC Supplement including HVHZ (within ESR-3889 and ESR-4042)

GUIDE SPECIFICATIONS

CSI Divisions: 03 16 00 - Concrete Anchors, 04 05 19.16 - Masonry Anchors and 05 05 19 - Post-Installed Concrete Anchors. Screw anchors shall be Screw-Bolt+ as supplied by DEWALT, Towson, MD. Anchors shall be installed in accordance with published instructions and the Authority Having Jurisdiction.

MATERIAL SPECIFICATIONS

	Anchor component	Specification
And	chor Body and hex washer head	Case hardened carbon steel
Plating	Standard zinc plated version	Zinc plating according to ASTM B633, SC1 Type III (Fe/Zn 5). Minimum plating requirements for Mild Service Condition
뿝	Mechanically galvanized version	Mechanically Galvanized zinc plating according to ASTM B695, Class 55

SECTION CONTENTS

General Information	1
Installation Specifications	2
Installation Instructions	2
Performance Data (ASD)	3
Strength Design Information	10
Design Strength Tables (SD)	16
Ordering Information	18



HEAD STYLES

· Hex Washer Head or Flat Head

ANCHOR MATERIALS

· Zinc plated carbon steel or mechanically galvanized plating

ANCHOR SIZE RANGE (TYP.)

• 1/4" through 3/4" diameters

SUITABLE BASE MATERIALS

- Normal-weight concrete
- · Lightweight concrete
- · Concrete over steel deck
- Grouted Concrete Masonry (CMU)
- Brick Masonry





CODE LISTED CC-ES ESR-3889 CONCRETE

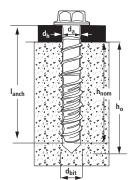
CODE LISTED ICC-ES ESR-4042 MASONRY





INSTALLATION SPECIFICATIONS

Screw-Bolt+ Anchor Detail (Hex Head Shown)



Nomenclature

 Diameter of Anchor dbit Diameter of Drill Bit Diameter of Clearance Hole Base Material Thickness. h_{nom} = Minimum Nominal Embedment

h₀ Minimum Hole Depth

Head Marking



Legend

Diameter and Length Identification Mark





Legend

Diameter and Length Identification Mark



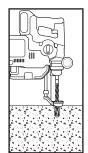
Installation Chariffortions for Corour Bolt . in Conserts on

allation Specificatio		THE DOILT III	oution oto ana	ouppicificitui	miloimation							
Anchor Property/	N-1-P			Nomi	nal Anchor Diameter	(inch)						
Setting Information	Notation	Units	1/4	3/8	1/2	5/8	3/4					
hor outside diameter	da (d)	in. (mm)	0.250 (6.4)	0.375 (9.5)	0.500 (12.7)	0.625 (15.9)	0.750 (19.1)					
ninal drill bit diameter (ANSI)	d _{bit}	in.	1/4	3/8	1/2	5/8	3/4					
imum diameter of hole trance in fixture	dн	in. (mm)	11/32 (8.7)	1/2 (12.7)	5/8 (15.9)	3/4 (19.1)	7/8 (22.2)					
imum embedment depth ¹	h _{nom}	in. (mm)	1 (25)	2-1/2 (64)	2-1/2 (64)							
imum hole depth	h₀	in. (mm)			h _{nom} + 3/8 (9.5)							
imum member thickness	h _{min}	in. (mm)		h _{nom} + 2 (51)								
imum edge distance	C _{min}	in. (mm)	1-1/2 (38)	1-1/2 (38)	1-3/4 (44)	1-3/4 (44)	1-3/4 (44)					
imum spacing	S _{min}	in. (mm)	1-1/2 (38)	2 (51)	2-3/4 (70)	2-3/4 (70)	3 (76)					
manual installation torque	T _{inst,max}	ftlbf. (N-m)	19 (26)	25 (34)	45 (61)	60 (81)	70 (95)					
(impact wrench power que)	T _{impact,max}	ftlbf. (N-m)	150 (203)	300 (407)	300 (407)	700 (950)	700 (950)					
Impact wrench socket size	-	in.	7/16	9/16	3/4	15/16	1-1/8					
Maximum head height	-	in.	21/64	3/8	31/64	37/64	43/64					
Maximum washer diameter	-	in.	37/64	3/4	1-1/16	1-1/8	1-13/32					
Driver Size	-	in.	T-30	T-50	T-55	-	-					
Max head height	-	in.	13/64	21/64	11/32	-	-					
Max head diameter	-	in.	17/32	57/64	1	-	-					
Countersunk angle	-	in.	82	82	82	-	-					
ctive tensile stress area ew anchor body)	Ase	in²	0.045	0.094	0.176	0.274	0.399					
imum ultimate strength	futa	psi	100,000	105,000	115,000	95,000	95,000					
imum yield strength	fy	psi	80,000	84,000	92,000	76,000	76,000					
	hor outside diameter ninal drill bit diameter (ANSI) imum diameter of hole rance in fixture imum embedment depth¹ imum hole depth imum member thickness imum edge distance imum spacing t manual installation torque t impact wrench power que) Impact wrench socket size Maximum head height Maximum washer diameter Driver Size Max head height Max head diameter Countersunk angle ctive tensile stress area ew anchor body) imum ultimate strength imum yield strength	hor outside diameter (ANSI) hor outside diameter (ANSI) hor diameter of hole dimum diameter of hole dimum diameter of hole dimum embedment depth hor dimum hole depth hor dimum member thickness homin dimum edge distance Comin dimum spacing Somin Timpact wrench power que limpact wrench power que limpact wrench socket size Maximum head height Maximum washer diameter Driver Size Max head height Max head diameter Countersunk angle ctive tensile stress area ew anchor body) futa imum yield strength futa fire diameter fy	hor outside diameter ANSI) dimum diameter of hole imum diameter of hole imum diameter of hole imum embedment depth hor in. imum hole depth hor in. imum member thickness hmin in. imum edge distance cmin in. (mm) imum spacing smin in. (mm) trustmax trustmax trustmax ftlbf. (N-m) trustmax ftlbf. (N-m) trustmax manual installation torque trustmax ftlbf. (N-m) trustmax t	Automotion	Notation Units 1/4 3/8	Notation Units 1/4 3/8 1/2	Setting Information					

See Strength Design Information for installation specifications in strict accordance with ICC-ES ESR-3889.

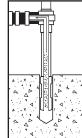
INSTALLATION INSTRUCTIONS

Installation Instructions for Screw-Bolt+ (Hex Head Version Illustrated, Flat Head Version Not Shown)



Step 1

Using the proper drill bit size, drill a hole into the base material to the required depth. The tolerances of the drill bit used should meet the requirements of ANSI standard B212.15



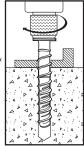
Step 2 Remove dust and debris from hole during

drilling (e.g. dust extractor, hollow bit) or following drilling (e.g. suction, forced air) to extract loose particles created during drilling.



Step 3

Select a torque wrench or powered impact wrench and do not exceed the maximum torque, Tinst,max or Timpact,max respectively for the selected anchor diameter and embedment. Attach an appropriate sized hex socket/driver to the impact wrench. Mount the screw anchor head into the socket.



Step 4

Drive the anchor into the hole until the head of the anchor comes into contact with the fixture. The anchor must be snug after installation. Do not spin the hex socket off the anchor to disengage.

^{1.} See load capacities for Screw-Bolt+ in normal weight concrete for additional nominal embedment depths.



PERFORMANCE DATA (ASD)

Ultimate Load Capacities for Screw-Bolt+ in Normal-Weight Concrete^{1,2}

	Minimum				Minim	um Concrete (Compressive S	trength			
Nominal Anchor	Nominal Embedment	f'c = 2, (17.3	500 psi MPa)	f'c = 3, (20.7	,000 psi MPa)		,000 psi MPa)		000 psi MPa)		000 psi MPa)
Diameter in.	Depth in. (mm)	Tension lbs (kN)	Shear Ibs (kN)								
	1	1,325	1,660	1,400	1,755	1,530	1,910	1,725	2,080	1,725	2,080
	(25)	(5.9)	(7.4)	(6.2)	(7.8)	(6.8)	(8.5)	(7.7)	(9.3)	(7.7)	(9.3)
1/4	1-5/8	2,835	1,660	2,995	1,755	3,265	1,910	3,265	2,080	3,265	2,080
	(41)	(12.6)	(7.4)	(13.3)	(7.8)	(14.5)	(8.5)	(14.5)	(9.3)	(14.5)	(9.3)
	2-1/2	3,650	2,025	3,855	2,140	4,200	2,335	4,270	2,545	4,270	2,545
	(64)	(16.2)	(9.0)	(17.1)	(9.5)	(18.7)	(10.4)	(19.0)	(11.3)	(19.0)	(11.3)
	1-1/2 (38)			2,880 (12.8)	3,890 (17.3)	3,330 (14.8)	4,490 (20.0)	4,075 (18.1)	5,500 (24.5)	4,075 (18.1)	6,355 (28.3)
	2	3,670	4,320	4,020	4,735	4,645	5,465	4,725	6,345	5,455	6,345
	(51)	(16.3)	(19.2)	(17.9)	(21.1)	(20.7)	(24.3)	(21.0)	(28.2)	(24.3)	(28.2)
3/8	2-1/2	5,175	4,320	5,670	4,740	6,410	5,460	6,456	6,340	7,420	6,340
	(64)	(23.0)	(19.2)	(25.2)	(21.1)	(28.5)	(24.3)	(28.7)	(28.2)	(33.0)	(28.2)
	3-1/4	7,420	6,325	8,130	6,930	9,065	8,000	9,065	8,565	10,350	8,565
	(83)	(33.0)	(28.1)	(36.2)	(30.8)	(40.3)	(35.6)	(40.3)	(38.1)	(46.0)	(38.1)
	4-1/2	10,905	6,325	11,945	6,930	13,795	8,000	15,075	8,565	15,075	8,565
	(114)	(48.5)	(28.1)	(53.1)	(30.8)	(61.4)	(35.6)	(67.1)	(38.1)	(67.1)	(38.1)
	1-3/4	2,840	5,985	3,115	6,555	3,595	7,570	4,400	9,270	4,400	10,705
	(44)	(12.6)	(26.6)	(13.9)	(29.2)	(16.0)	(33.7)	(19.6)	(41.2)	(19.6)	(47.6)
	2-1/2	6,680	8,035	7,320	8,800	8,450	10,160	8,450	11,545	8,450	11,545
	(64)	(29.7)	(35.7)	(32.6)	(39.1)	(37.6)	(45.2)	(37.6)	(51.4)	(37.6)	(51.4)
1/2	3	8,560	8,040	9,375	8,800	10,750	10,160	10,750	11,540	10,750	11,540
	(76)	(38.0)	(35.8)	(41.7)	(39.1)	(47.8)	(45.2)	(47.8)	(51.3)	(47.8)	(51.3)
	4-1/4	13,260	9,395	14,525	10,290	16,480	11,885	16,480	13,520	16,480	13,520
	(108)	(59.0)	(41.8)	(64.6)	(45.8)	(73.3)	(52.9)	(73.3)	(60.1)	(73.3)	(60.1)
	5-1/2	15,730	9,395	17,235	10,290	19,900	11,885	21,310	13,520	21,310	13,520
	(140)	(70.0)	(41.8)	(76.7)	(45.8)	(88.5)	(52.9)	(94.8)	(60.1)	(94.8)	(60.1)
	2-1/2	5,735	10,615	6,285	11,630	7,255	13,425	8,885	16,445	8,885	17,170
	(64)	(25.5)	(47.2)	(28.0)	(51.7)	(32.3)	(59.7)	(39.5)	(73.2)	(39.5)	(76.4)
	3-1/4	9,755	12,065	10,685	13,220	12,340	15,265	12,340	17,170	12,340	17,170
	(83)	(43.4)	(53.7)	(47.5)	(58.8)	(54.9)	(67.9)	(54.9)	(76.4)	(54.9)	(76.4)
5/8	4	11,770	12,060	12,890	13,220	14,880	15,260	15,325	17,180	16,600	17,180
	(102)	(52.4)	(53.6)	(57.3)	(58.8)	(66.2)	(67.9)	(68.2)	(76.4)	(73.8)	(76.4)
	5	14,455	13,675	15,830	14,980	18,280	17,295	19,295	19,485	22,280	19,485
	(127)	(64.3)	(60.8)	(70.4)	(66.6)	(81.3)	(76.9)	(85.8)	(86.7)	(99.1)	(86.7)
	6-1/4	20,520	13,675	22,475	14,980	25,955	17,295	31,785	19,485	31,785	19,485
	(159)	(91.3)	(60.8)	(100.0)	(66.6)	(115.5)	(76.9)	(141.4)	(86.7)	(141.4)	(86.7)
	2-1/2	6,035	11,615	6,610	12,725	7,635	14,690	9,350	17,995	9,350	20,775
	(64)	(26.8)	(51.7)	(29.4)	(56.6)	(34.0)	(65.3)	(41.6)	(80.0)	(41.6)	(92.4)
3/4	4-1/4	11,900	17,055	13,035	18,685	15,050	21,575	17,745	24,270	20,490	24,270
	(108)	(52.9)	(75.9)	(58.0)	(83.1)	(66.9)	(96.0)	(78.9)	(108.0)	(91.1)	(108.0)
O/ T	5	19,020	17,055	20,835	18,685	24,055	21,575	29,460	24,270	29,460	24,270
	(127)	(84.6)	(75.9)	(92.7)	(83.1)	(107.0)	(96.0)	(131.0)	(108.0)	(131.0)	(108.0)
	6-1/4	20,495	17,055	22,450	18,685	25,920	21,575	31,750	24,270	31,750	24,270
	(159)	(91.2)	(75.9)	(99.9)	(83.1)	(115.3)	(96.0)	(141.2)	(108.0)	(141.2)	(108.0)

^{1.} Tabulated load values are for anchors installed in uncracked concrete with no edge or spacing considerations. Concrete compressive strength must be at the specified minimum at the time of installation.

^{2.} Ultimate load capacities must be reduced by a minimum safety factor of 4.0 or greater to determine allowable working load.



Allowable Load Capacities for Screw-Bolt+ in Normal-Weight Concrete^{1,2,3,4,5}



	Minimum				Minim	um Concrete (Compressive S	trength			
Nominal	Nominal	f'c = 2,	500 psi	f'c = 3	,000 psi	f'c = 4	,000 psi	f'c = 6,	000 psi	f'c = 8,	000 psi
Anchor	Embedment	(17.3	MPa)	(20.7	MPa)	(27.6	MPa)	(41.4	MPa)	(55.2	MPa)
Diameter in.	Depth in. (mm)	Tension lbs (kN)	Shear lbs (kN)	Tension lbs (kN)	Shear Ibs (kN)	Tension lbs (kN)	Shear lbs (kN)	Tension lbs (kN)	Shear lbs (kN)	Tension lbs (kN)	Shear lbs (kN)
	1	330	415	350	440	385	480	430	520	430	520
	(25)	(1.5)	(1.8)	(1.6)	(2.0)	(1.7)	(2.1)	(1.9)	(2.3)	(1.9)	(2.3)
1/4	1-5/8	710	415	750	440	815	480	815	520	815	520
	(41)	(3.2)	(1.8)	(3.3)	(2.0)	(3.6)	(2.1)	(3.6)	(2.3)	(3.6)	(2.3)
	2-1/2	915	505	965	535	1,050	585	1,070	635	1,070	635
	(64)	(4.1)	(2.2)	(4.3)	(2.4)	(4.7)	(2.6)	(4.8)	(2.8)	(4.8)	(2.8)
	1-1/2	660	890	720	975	835	1,125	1,020	1,375	1,020	1,590
	(38)	(2.9)	(4.0)	(3.2)	(4.3)	(3.7)	(5.0)	(4.5)	(6.1)	(4.5)	(7.1)
	2	920	1,080	1,005	1,185	1,160	1,365	1,180	1,585	1,365	1,585
	(51)	(4.1)	(4.8)	(4.5)	(5.3)	(5.2)	(6.1)	(5.2)	(7.1)	(6.1)	(7.1)
3/8	2-1/2	1,295	1,080	1,415	1,185	1,600	1,365	1,615	1,585	1,855	1,585
	(64)	(5.8)	(4.8)	(6.3)	(5.3)	(7.1)	(6.1)	(7.2)	(7.1)	(8.3)	(7.1)
	3-1/4	1,855	1,580	2,035	1,735	2,265	2,000	2,265	2,140	2,590	2,140
	(83)	(8.3)	(7.0)	(9.1)	(7.7)	(10.1)	(8.9)	(10.1)	(9.5)	(11.5)	(9.5)
	4-1/2	2,725	1,580	2,985	1,735	3,450	2,000	3,770	2,140	3,770	2,140
	(114)	(12.1)	(7.0)	(13.3)	(7.7)	(15.3)	(8.9)	(16.8)	(9.5)	(16.8)	(9.5)
	1-3/4	710	1,495	780	1,640	900	1,895	1,100	2,320	1,100	2,675
	(44)	(3.2)	(6.7)	(3.5)	(7.3)	(4.0)	(8.4)	(4.9)	(10.3)	(4.9)	(11.9)
	2-1/2	1,670	2,010	1,830	2,200	2,115	2,540	2,115	2,885	2,115	2,885
	(64)	(7.4)	(8.9)	(8.1)	(9.8)	(9.4)	(11.3)	(9.4)	(12.8)	(9.4)	(12.8)
1/2	3	2,140	2,010	2,345	2,200	2,690	2,540	2,690	2,885	2,690	2,885
	(76)	(9.5)	(8.9)	(10.4)	(9.8)	(11.9)	(11.3)	(11.9)	(12.8)	(11.9)	(12.8)
	4-1/4	3,315	2,350	3,630	2,575	4,120	2,970	4,120	3,380	4,120	3,380
	(108)	(14.7)	(10.5)	(16.1)	(11.5)	(18.3)	(13.2)	(18.3)	(15.0)	(18.3)	(15.0)
	5-1/2	3,935	2,350	4,310	2,575	4,975	2,970	5,330	3,380	5,330	3,380
	(140)	(17.5)	(10.5)	(19.2)	(11.5)	(22.1)	(13.2)	(23.7)	(15.0)	(23.7)	(15.0)
	2-1/2	1,435	2,655	1,570	2,910	1,815	3,355	2,220	4,110	2,220	4,295
	(64)	(6.4)	(11.8)	(7.0)	(12.9)	(8.1)	(14.9)	(9.9)	(18.3)	(9.9)	(19.1)
	3-1/4	2,440	3,015	2,670	3,305	3,085	3,815	3,085	4,295	3,085	4,295
	(83)	(10.9)	(13.4)	(11.9)	(14.7)	(13.7)	(17.0)	(13.7)	(19.1)	(13.7)	(19.1)
5/8	4	2,940	3,015	3,225	3,305	3,720	3,815	3,830	4,295	4,150	4,295
	(102)	(13.1)	(13.4)	(14.3)	(14.7)	(16.5)	(16.9)	(17.0)	(19.1)	(18.5)	(19.1)
	5	3,615	3,420	3,960	3,745	4,570	4,325	4,825	4,870	5,570	4,870
	(127)	(16.1)	(15.2)	(17.6)	(16.7)	(20.3)	(19.2)	(21.5)	(21.7)	(24.8)	(21.7)
	6-1/4	5,130	3,420	5,620	3,745	6,490	4,325	7,945	4,870	7,945	4,870
	(159)	(22.8)	(15.2)	(25.0)	(16.7)	(28.9)	(19.2)	(35.3)	(21.7)	(35.3)	(21.7)
	2-1/2	1,510	2,905	1,655	3,180	1,910	3,675	2,340	4,500	2,340	5,195
	(64)	(6.7)	(12.9)	(7.4)	(14.1)	(8.5)	(16.3)	(10.4)	(20.0)	(10.4)	(23.1)
3/4	4-1/4	2,975	4,265	3,260	4,670	3,765	5,395	4,435	6,070	5,125	6,070
	(108)	(13.2)	(19.0)	(14.5)	(20.8)	(16.7)	(24.0)	(19.7)	(27.0)	(22.8)	(27.0)
5/7	5	4,755	4,265	5,210	4,670	6,015	5,395	7,365	6,070	7,365	6,070
	(127)	(21.2)	(19.0)	(23.2)	(20.8)	(26.8)	(24.0)	(32.8)	(27.0)	(32.8)	(27.0)
	6-1/4	5,125	4,265	5,615	4,670	6,480	5,395	7,940	6,070	7,940	6,070
	(159)	(22.8)	(19.0)	(25.0)	(20.8)	(28.8)	(24.0)	(35.3)	(27.0)	(35.3)	(27.0)

- 1. Tabulated load values are for anchors installed in uncracked concrete. Concrete compressive strength must be at the specified minimum at the time of installation.
- 2. Allowable load capacities are calculated using an applied safety factor of 4.0 to average ultimate load capacities.
- 3. Allowable load capacities must be multiplied by reduction factors when anchor spacing or edge distances are less than critical distances.
- 4. Linear interpolation may be used to determine allowable loads for intermediate embedments and compressive strengths.
- 5. For lightweight concrete multiply tabulated allowable load values by a reduction factor of 0.60.



LOAD ADJUSTMENT FACTORS FOR NORMAL-WEIGHT CONCRETE

Edge Distance Reduction Factors - Tension (F_{NC})

Dia	Diameter (in) 1/4					3/8		(= NO)	1/2				5/8					3/4					
En	Nominal nbedment hnom (in)	1	1-5/8	2-1/2	1-1/2	2	2-1/2	3-1/4	4-1/2	1-3/4	2-1/2	3	4-1/4	5-1/2	2-1/2	3-1/4	4	5	6-1/4	2-1/2	4-1/4	5	6-1/4
	lin. Edge stance cmin (in)	1-1/2	1-1/2	1-1/2	1-1/2	1-1/2	1-1/2	1-1/2	1-1/2	1-3/4	1-3/4	1-3/4	1-3/4	1-3/4	1-3/4	1-3/4	1-3/4	1-3/4	1-3/4	1-3/4	1-3/4	1-3/4	1-3/4
	1-1/2	1.00	0.77	0.64	0.85	0.74	0.67	0.59	0.55	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	1-3/4	1.00	0.83	0.67	0.93	0.79	0.71	0.62	0.57	0.87	0.71	0.65	0.58	0.54	0.73	0.65	0.60	0.56	0.53	0.73	0.59	0.56	0.53
	2	1.00	0.88	0.71	1.00	0.84	0.76	0.65	0.59	0.94	0.76	0.68	0.60	0.56	0.78	0.68	0.63	0.58	0.54	0.78	0.61	0.58	0.54
	2-1/4	1.00	0.94	0.75	1.00	0.89	0.80	0.68	0.61	1.00	0.80	0.71	0.63	0.57	0.82	0.71	0.65	0.60	0.56	0.82	0.63	0.60	0.56
	2-1/2	1.00	1.00	0.78	1.00	0.95	0.84	0.71	0.63	1.00	0.84	0.74	0.65	0.59	0.87	0.75	0.68	0.62	0.57	0.87	0.66	0.62	0.57
	2-3/4	1.00	1.00	0.82	1.00	1.00	0.88	0.74	0.65	1.00	0.88	0.77	0.67	0.61	0.91	0.78	0.70	0.64	0.59	0.91	0.68	0.64	0.59
	3	1.00	1.00	0.86	1.00	1.00	0.92	0.77	0.67	1.00	0.92	0.81	0.69	0.62	0.96	0.81	0.73	0.66	0.60	0.96	0.70	0.66	0.60
es (s	3-1/2	1.00	1.00	0.93	1.00	1.00	1.00	0.83	0.71	1.00	1.00	0.87	0.74	0.65	1.00	0.87	0.78	0.69	0.63	1.00	0.75	0.69	0.63
(inches)	4	1.00	1.00	1.00	1.00	1.00	1.00	0.88	0.75	1.00	1.00	0.94	0.78	0.69	1.00	0.94	0.83	0.73	0.66	1.00	0.79	0.73	0.66
	4-1/2	1.00	1.00	1.00	1.00	1.00	1.00	0.94	0.79	1.00	1.00	1.00	0.82	0.72	1.00	1.00	0.88	0.77	0.69	1.00	0.84	0.77	0.69
auc	5	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.84	1.00	1.00	1.00	0.87	0.75	1.00	1.00	0.93	0.81	0.72	1.00	0.89	0.81	0.72
Edge Distance	5-1/2	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.88	1.00	1.00	1.00	0.91	0.79	1.00	1.00	0.98	0.85	0.75	1.00	0.93	0.85	0.75
ge	6	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.92	1.00	1.00	1.00	0.96	0.82	1.00	1.00	1.00	0.89	0.78	1.00	0.98	0.89	0.78
圖	6-1/2	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.96	1.00	1.00	1.00	1.00	0.85	1.00	1.00	1.00	0.92	0.81	1.00	1.00	0.92	0.81
	7	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.88	1.00	1.00	1.00	0.96	0.84	1.00	1.00	0.96	0.84
	7-1/2	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.92	1.00	1.00	1.00	1.00	0.87	1.00	1.00	1.00	0.87
	8	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00	1.00	0.90	1.00	1.00	1.00	0.90
	8-1/2	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.98	1.00	1.00	1.00	1.00	0.93	1.00	1.00	1.00	0.93
	9	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.96	1.00	1.00	1.00	0.96
	9-1/2	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.99	1.00	1.00	1.00	0.99
	10	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Spacing Reduction Factors - Tension (F_{NS})

			duction Factors - Tension (F _{NS})																				
_	meter (in)		1/4				3/8					1/2					5/8			3/4			
Em	lominal bedment nom (in)	1	1-5/8	2-1/2	1-1/2	2	2-1/2	3-1/4	4-1/2	1-3/4	2-1/2	3	4-1/4	5-1/2	2-1/2	3-1/4	4	5	6-1/4	2-1/2	4-1/4	5	6-1/4
	linimum acing smin (in)	1-1/2	1-1/2	1-1/2	2	2	2	2	2	2-3/4	2-3/4	2-3/4	2-3/4	2-3/4	2-3/4	2-3/4	2-3/4	2-3/4	2-3/4	3	3	3	3
	1-1/2	0.89	0.73	0.66	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	1-3/4	0.94	0.77	0.68	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	2	1.00	0.80	0.70	0.88	0.77	0.71	0.67	0.63	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	2-1/4	1.00	0.83	0.72	0.93	0.80	0.74	0.69	0.64	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	2-1/2	1.00	0.86	0.74	0.97	0.83	0.76	0.70	0.65	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	2-3/4	1.00	0.89	0.76	1.00	0.86	0.78	0.72	0.66	0.92	0.78	0.74	0.67	0.64	0.80	0.73	0.69	0.65	0.63	-	-	-	-
	3	1.00	0.92	0.78	1.00	0.89	0.80	0.74	0.67	0.95	0.80	0.75	0.68	0.65	0.83	0.74	0.70	0.66	0.64	0.83	0.69	0.66	0.64
	3-1/2	1.00	0.99	0.82	1.00	0.94	0.85	0.77	0.70	1.00	0.85	0.79	0.71	0.67	0.88	0.78	0.73	0.68	0.65	0.88	0.71	0.68	0.65
	4	1.00	1.00	0.86	1.00	1.00	0.89	0.80	0.72	1.00	0.89	0.82	0.73	0.68	0.92	0.81	0.75	0.70	0.67	0.93	0.74	0.71	0.67
	4-1/2	1.00	1.00	0.90	1.00	1.00	0.93	0.83	0.74	1.00	0.93	0.86	0.75	0.70	0.97	0.85	0.78	0.72	0.68	0.97	0.76	0.73	0.69
(Si	5	1.00	1.00	0.94	1.00	1.00	0.98	0.86	0.76	1.00	0.98	0.89	0.78	0.72	1.00	0.88	0.81	0.75	0.70	1.00	0.79	0.75	0.70
(inches)	5-1/2	1.00	1.00	0.97	1.00	1.00	1.00	0.89	0.78	1.00	1.00	0.93	0.80	0.74	1.00	0.92	0.83	0.77	0.72	1.00	0.81	0.77	0.72
	6	1.00	1.00	1.00	1.00	1.00	1.00	0.93	0.81	1.00	1.00	0.96	0.82	0.75	1.00	0.95	0.86	0.79	0.73	1.00	0.84	0.79	0.73
Distance	6-1/2	1.00	1.00	1.00	1.00	1.00	1.00	0.96	0.83	1.00	1.00	1.00	0.85	0.77	1.00	0.98	0.89	0.81	0.75	1.00	0.86	0.81	0.75
ists	7	1.00	1.00	1.00	1.00	1.00	1.00	0.99	0.85	1.00	1.00	1.00	0.87	0.79	1.00	1.00	0.91	0.83	0.76	1.00	0.89	0.83	0.77
	7-1/2	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.87	1.00	1.00	1.00	0.90	0.81	1.00	1.00	0.94	0.85	0.78	1.00	0.91	0.85	0.78
Spacing	8	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.90	1.00	1.00	1.00	0.92	0.83	1.00	1.00	0.97	0.87	0.80	1.00	0.94	0.87	0.80
Sp	8-1/2	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.92	1.00	1.00	1.00	0.94	0.84	1.00	1.00	0.99	0.89	0.81	1.00	0.96	0.89	0.81
	9	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.94	1.00	1.00	1.00	0.97	0.86	1.00	1.00	1.00	0.91	0.83	1.00	0.99	0.91	0.83
	9-1/2	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.96	1.00	1.00	1.00	0.99	0.88	1.00	1.00	1.00	0.93	0.84	1.00	1.00	0.93	0.85
	10	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.98	1.00	1.00	1.00	1.00	0.90	1.00	1.00	1.00	0.95	0.86	1.00	1.00	0.95	0.86
	10-1/2	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	1.00	1.00	1.00	0.97	0.88	1.00	1.00	0.97	0.88
	11	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.93	1.00	1.00	1.00	0.99	0.89	1.00	1.00	0.99	0.89
	11-1/2	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	1.00	1.00	0.91	1.00	1.00	1.00	0.91
	12	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.97	1.00	1.00	1.00	1.00	0.92	1.00	1.00	1.00	0.93
	13	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.96	1.00	1.00	1.00	0.96
	14	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.99	1.00	1.00	1.00	0.99
	15	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00



Edge Distance Reduction Factors - Shear (Fvc)

Dia	Diameter (in) 1/4						3/8			1/2				5/8					3/4				
En	Nominal nbedment hom (in)	1	1-5/8	2-1/2	1-1/2	2	2-1/2	3-1/4	4-1/2	1-3/4	2-1/2	3	4-1/4	5-1/2	2-1/2	3-1/4	4	5	6-1/4	2-1/2	4-1/4	5	6-1/4
	lin. Edge Distance Cmin(in)	1-1/2	1-1/2	1-1/2	1-1/2	1-1/2	1-1/2	1-1/2	1-1/2	1-3/4	1-3/4	1-3/4	1-3/4	1-3/4	1-3/4	1-3/4	1-3/4	1-3/4	1-3/4	1-3/4	1-3/4	1-3/4	1-3/4
	1-1/2	0.58	0.63	0.59	0.40	0.37	0.39	0.31	0.32	-	-	-	-	-	-	-	-	-	-	-	-	-	-
	1-3/4	0.68	0.73	0.69	0.46	0.43	0.45	0.36	0.38	0.35	0.31	0.36	0.30	0.31	0.27	0.26	0.32	0.25	0.26	0.26	0.22	0.22	0.23
	2	0.78	0.84	0.78	0.53	0.49	0.52	0.41	0.43	0.41	0.35	0.41	0.35	0.36	0.30	0.29	0.37	0.29	0.30	0.30	0.25	0.26	0.27
	2-1/4	0.87	0.94	0.88	0.59	0.55	0.58	0.46	0.48	0.46	0.40	0.46	0.39	0.40	0.34	0.33	0.41	0.32	0.33	0.33	0.28	0.29	0.30
	2-1/2	0.97	1.00	0.98	0.66	0.61	0.64	0.51	0.54	0.51	0.44	0.51	0.43	0.45	0.38	0.36	0.46	0.36	0.37	0.37	0.31	0.32	0.33
(Se	2-3/4	1.00	1.00	1.00	0.73	0.67	0.71	0.56	0.59	0.56	0.49	0.56	0.48	0.49	0.42	0.40	0.51	0.40	0.41	0.41	0.34	0.35	0.37
(inches)	3	1.00	1.00	1.00	0.79	0.73	0.77	0.61	0.64	0.61	0.53	0.61	0.52	0.54	0.46	0.44	0.55	0.43	0.45	0.44	0.38	0.39	0.40
	3-1/2	1.00	1.00	1.00	0.92	0.85	0.90	0.72	0.75	0.71	0.62	0.72	0.61	0.63	0.53	0.51	0.64	0.50	0.52	0.52	0.44	0.45	0.47
Distance	4	1.00	1.00	1.00	1.00	0.97	1.00	0.82	0.86	0.81	0.71	0.82	0.69	0.72	0.61	0.58	0.74	0.57	0.59	0.59	0.50	0.51	0.53
ist	4-1/2	1.00	1.00	1.00	1.00	1.00	1.00	0.92	0.97	0.91	0.80	0.92	0.78	0.81	0.68	0.66	0.83	0.65	0.67	0.67	0.56	0.58	0.60
	5	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.89	1.00	0.87	0.90	0.76	0.73	0.92	0.72	0.74	0.74	0.63	0.64	0.66
Edge	5-1/2	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.97	1.00	0.95	0.99	0.84	0.80	1.00	0.79	0.82	0.82	0.69	0.71	0.73
	6	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.88	1.00	0.86	0.89	0.89	0.75	0.77	0.80
	6-1/2	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.99	0.95	1.00	0.93	0.97	0.96	0.81	0.84	0.86
	7	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.88	0.90	0.93
	7-1/2	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.94	0.96	1.00
	8	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00

Spacing Reduction Factors - Shear (Fvs)

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(in)
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ing s _{min}
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| ing Smin | | | | | | ,_ | 0 1/4 | 4-1/2

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 | 5-1/2 | 2-1/2 | 3-1/4 | 4 | 5
 | 6-1/4 | 2-1/2 | 4-1/4 | 5 | 6-1/4 |
| | 1-1/2 | 1-1/2 | 1-1/2 | 2 | 2 | 2 | 2 | 2

 | 2-3/4 | 2-3/4
 | 2-3/4 | 2-3/4
 | 2-3/4 | 2-3/4 | 2-3/4 | 2-3/4 | 2-3/4
 | 2-3/4 | 3 | 3 | 3 | 3 |
| 1-1/2 | 0.60 | 0.60 | 0.60 | - | - | - | - | -

 | - | -
 | - | -
 | - | - | - | - | -
 | - | - | - | - | - |
| 1-3/4 | 0.61 | 0.62 | 0.61 | - | - | - | - | -

 | - | -
 | - | -
 | - | - | - | - | -
 | - | - | - | - | - |
| 2 | 0.63 | 0.64 | 0.63 | 0.59 | 0.58 | 0.59 | 0.57 | 0.57

 | - | -
 | - | -
 | - | - | - | - | -
 | - | - | - | - | - |
| 2-1/4 | 0.65 | 0.66 | 0.65 | 0.60 | 0.59 | 0.60 | 0.58 | 0.58

 | - | -
 | - | -
 | - | - | - | - | -
 | - | - | - | - | - |
| 2-1/2 | 0.66 | 0.67 | 0.66 | 0.61 | 0.60 | 0.61 | 0.59 | 0.59

 | - | -
 | - | -
 | - | - | - | - | -
 | - | - | - | - | - |
| 2-3/4 | 0.68 | 0.69 | 0.68 | 0.62 | 0.61 | 0.62 | 0.59 | 0.60

 | 0.59 | 0.58
 | 0.59 | 0.58
 | 0.58 | 0.57 | 0.57 | 0.58 | 0.57
 | 0.57 | - | - | - | - |
| 3 | 0.69 | 0.71 | 0.70 | 0.63 | 0.62 | 0.63 | 0.60 | 0.61

 | 0.60 | 0.59
 | 0.60 | 0.59
 | 0.59 | 0.58 | 0.57 | 0.59 | 0.57
 | 0.57 | 0.57 | 0.56 | 0.56 | 0.57 |
| 3-1/2 | 0.73 | 0.74 | 0.73 | 0.65 | 0.64 | 0.65 | 0.62 | 0.63

 | 0.62 | 0.60
 | 0.62 | 0.60
 | 0.60 | 0.59 | 0.59 | 0.61 | 0.58
 | 0.59 | 0.59 | 0.57 | 0.57 | 0.58 |
| 4 | 0.76 | 0.78 | 0.76 | 0.68 | 0.66 | 0.67 | 0.64 | 0.64

 | 0.64 | 0.62
 | 0.64 | 0.62
 | 0.62 | 0.60 | 0.60 | 0.62 | 0.60
 | 0.60 | 0.60 | 0.58 | 0.59 | 0.59 |
| 4-1/2 | 0.79 | 0.81 | 0.79 | 0.70 | 0.68 | 0.69 | 0.65 | 0.66

 | 0.65 | 0.63
 | 0.65 | 0.63
 | 0.63 | 0.61 | 0.61 | 0.64 | 0.61
 | 0.61 | 0.61 | 0.59 | 0.60 | 0.60 |
| 5 | 0.82 | 0.85 | 0.83 | 0.72 | 0.70 | 0.71 | 0.67 | 0.68

 | 0.67 | 0.65
 | 0.67 | 0.64
 | 0.65 | 0.63 | 0.62 | 0.65 | 0.62
 | 0.62 | 0.62 | 0.60 | 0.61 | 0.61 |
| 5-1/2 | 0.86 | 0.88 | 0.86 | 0.74 | 0.72 | 0.74 | 0.69 | 0.70

 | 0.69 | 0.66
 | 0.69 | 0.66
 | 0.66 | 0.64 | 0.63 | 0.67 | 0.63
 | 0.64 | 0.64 | 0.61 | 0.62 | 0.62 |
| 6 | 0.89 | 0.92 | 0.89 | 0.76 | 0.74 | 0.76 | 0.70 | 0.71

 | 0.70 | 0.68
 | 0.70 | 0.67
 | 0.68 | 0.65 | 0.65 | 0.68 | 0.64
 | 0.65 | 0.65 | 0.63 | 0.63 | 0.63 |
| 6-1/2 | 0.92 | 0.95 | 0.92 | 0.79 | 0.76 | 0.78 | 0.72 | 0.73

 | 0.72 | 0.69
 | 0.72 | 0.69
 | 0.69 | 0.66 | 0.66 | 0.70 | 0.66
 | 0.66 | 0.66 | 0.64 | 0.64 | 0.64 |
| 7 | 0.95 | 0.99 | 0.96 | 0.81 | 0.78 | 0.80 | 0.74 | 0.75

 | 0.74 | 0.71
 | 0.74 | 0.70
 | 0.71 | 0.68 | 0.67 | 0.71 | 0.67
 | 0.67 | 0.67 | 0.65 | 0.65 | 0.66 |
| 7-1/2 | 0.99 | 1.00 | 0.99 | 0.83 | 0.80 | 0.82 | 0.76 | 0.77

 | 0.75 | 0.72
 | 0.76 | 0.72
 | 0.72 | 0.69 | 0.68 | 0.73 | 0.68
 | 0.69 | 0.69 | 0.66 | 0.66 | 0.67 |
| 8 | 1.00 | 1.00 | 1.00 | 0.85 | 0.82 | 0.84 | 0.77 | 0.79

 | 0.77 | 0.74
 | 0.77 | 0.73
 | 0.74 | 0.70 | 0.69 | 0.75 | 0.69
 | 0.70 | 0.70 | 0.67 | 0.67 | 0.68 |
| 9 | 1.00 | 1.00 | 1.00 | 0.90 | 0.87 | 0.89 | 0.81 | 0.82

 | 0.80 | 0.77
 | 0.81 | 0.76
 | 0.77 | 0.73 | 0.72 | 0.78 | 0.72
 | 0.72 | 0.72 | 0.69 | 0.69 | 0.70 |
| 10 | 1.00 | 1.00 | 1.00 | 0.94 | 0.91 | 0.93 | 0.84 | 0.86

 | 0.84 | 0.80
 | 0.84 | 0.79
 | 0.80 | 0.75 | 0.74 | 0.81 | 0.74
 | 0.75 | 0.75 | 0.71 | 0.71 | 0.72 |
| 11 | 1.00 | 1.00 | 1.00 | 0.98 | 0.95 | 0.97 | 0.87 | 0.89

 | 0.87 | 0.82
 | 0.87 | 0.82
 | 0.83 | 0.78 | 0.77 | 0.84 | 0.76
 | 0.77 | 0.77 | 0.73 | 0.74 | 0.74 |
| 12 | 1.00 | 1.00 | 1.00 | 1.00 | 0.99 | 1.00 | 0.91 | 0.93

 | 0.91 | 0.85
 | 0.91 | 0.85
 | 0.86 | 0.80 | 0.79 | 0.87 | 0.79
 | 0.80 | 0.80 | 0.75 | 0.76 | 0.77 |
| 13 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.94 | 0.96

 | 0.94 | 0.88
 | 0.94 | 0.88
 | 0.89 | 0.83 | 0.82 | 0.90 | 0.81
 | 0.82 | 0.82 | 0.77 | 0.78 | 0.79 |
| 14 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.98 | 1.00

 | 0.97 | 0.91
 | 0.98 | 0.90
 | 0.92 | 0.85 | 0.84 | 0.93 | 0.84
 | 0.85 | 0.85 | 0.79 | 0.80 | 0.81 |
| 15 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00

 | 1.00 | 0.94
 | 1.00 | 0.93
 | 0.95 | 0.88 | 0.86 | 0.96 | 0.86
 | 0.87 | 0.87 | 0.81 | 0.82 | 0.83 |
| 16 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00

 | 1.00 | 0.97
 | 1.00 | 0.96
 | 0.98 | 0.91 | 0.89 | 0.99 | 0.88
 | 0.90 | 0.90 | 0.83 | 0.84 | 0.85 |
| 17 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00

 | 1.00 | 1.00
 | 1.00 | 0.99
 | 1.00 | 0.93 | 0.91 | 1.00 | 0.91
 | 0.92 | 0.92 | 0.86 | 0.86 | 0.88 |
| 18 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00

 | 1.00 | 1.00
 | 1.00 | 1.00
 | 1.00 | 0.96 | 0.94 | 1.00 | 0.93
 | 0.95 | 0.94 | 0.88 | 0.89 | 0.90 |
| 19 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00

 | 1.00 | 1.00
 | 1.00 | 1.00
 | 1.00 | 0.98 | 0.96 | 1.00 | 0.95
 | 0.97 | 0.97 | 0.90 | 0.91 | 0.92 |
| 20 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00

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 | 1.00 | 1.00 | 0.99 | 1.00 | 0.98
 | 1.00 | 0.99 | 0.92 | 0.93 | 0.94 |
| 21 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00

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DEWALT. ANCHORS & FASTENERS

Allowable Screw-Bolt+ Tension and Shear Load Capacities Installed into the face of Grout-Filled Concrete Masonry Units^{1,2,3,4,5,6,7,8,9}



				Tension Load				
Anchor Diameter,	Minimum Embedment	Allowable Load		Spacing Distance,	s	Edge or End Di Screw-Bolt+ Inst	stance, c2 or c1 (se alled into Grouted (Wall detail)	e Illustration of Concrete Masonry
d in.	h _{nom} in. (mm)	lbs (kN)	Critical Distance, sa in. (mm)	Minimum Distance, Smin in. (mm)	Allowable Load Factor at Smin	Critical Distance, cor in. (mm)	Minimum Distance, Cmin in. (mm)	Allowable Load Factor at cmin
1/4	1-5/8 (41) 2-1/2 (64)	315 (1.4) 605 (2.7)	4 (102)	2 (51)	1.00 (no reduction)	3-3/4 (95)	1-1/4 (32)	0.60
3/8	2 (51) 3-1/4 (83)	450 (2.0) 1,085 (4.8)	6 (152)	3 (76)	1.00 (no reduction)	6 (152)	1-1/2 (38)	0.70
1/2	2-1/2 (64) 4-1/4 (108)	610 (2.7) 1,190 (5.3)	8 (203)	4 (102)	1.00 (no reduction)	8 (203)	2-5/8 (67)	0.75
5/8	3-1/4 (83) 5 (127)	880 (3.9) 1,270 (5.6)	10 (254)	4 (102)	1.00 (no reduction)	10 (254)	3-3/8 (88)	0.90
3/4	4 (102) 6-1/4 (159)	1,150 (5.1) 1,355 (6.0)	12 (305)	4 (102)	1.00 (no reduction)	12 (305)	4 (102)	1.00 (no reduction)

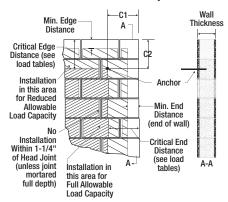
Shear Load

					Sileai Luau					
				Sp	acing Distance	e, s			(see Illustration Concrete Mason	
Anchor	Minimum Embedment	Allowable Load at Cor and Sor	Allowable Load at cor and sor						Allowable Loa	d Factor at cmin
Diameter, d in.	h _{nom} in. (mm)	Direction 1 & 2 lbs ⁹ (kN)	Direction 3 & 4 lbs ⁹ (kN)	Critical Distance, ser in. (mm)	Minimum Distance, Smin in. (mm)	Allowable Load Factor at Smin	Critical Distance, car in. (mm)	Minimum Distance, Cmin in. (mm)	Load Perpendicular to Edge or End (Direction 1 & 2)	Load Perpendicular to Edge or End (Direction 3 & 4)°
1/4	1-5/8 (41) 2-1/2 (64)	400 (1.8) 505 (2.2)	400 (1.8) 505 (2.2)	4 (102)	2 (51)	1.00 (no reduction)	3-3/4 (95)	1-1/4 (32)	0.35	1.00 (no reduction)
3/8	2 (51) 3-1/4 (83)	815 (3.6) 935 (4.2)	815 (3.6) 935 (4.2)	6 (152)	3 (76)	1.00 (no reduction)	6 (152)	1-1/2 (38)	0.27	1.00 (no reduction)
1/2	2-1/2 (64) 4-1/4 (108)	1,380 (6.1) 2,180 (9.7)	1,380 (6.1) 2,180 (9.7)	8 (203)	4 (102)	1.00 (no reduction)	8 (203)	2-5/8 (67)	0.20	1.00 (no reduction)
5/8	3-1/4 (83) 5 (127)	2,090 (9.3) 2,640 (11.7)	2,225 (9.9) 2,640 (11.7)	10 (254)	4 (102)	1.00 (no reduction)	10 (254)	3-3/8 (86)	0.23	1.00 (no reduction)
3/4	4 (102) 6-1/4 (159)	2,800 (12.5) 3,100 (13.8)	3,330 (14.8) 3,685 (16.4)	12 (305)	4 (102)	1.00 (no reduction)	12 (305)	4 (102)	0.25	1.00 (no reduction)

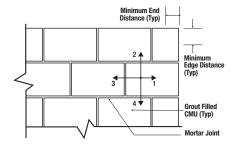
For SI: 1 inch = 25.4 mm; 1 lbs = 0.0044 kN, 1 psi = 0.006894 MPa.

- 1. All values are for anchors installed in fully grouted concrete masonry wall construction with materials meeting minimum compressive strength, f'm, of 1,500 psi (10.3 MPa). Concrete masonry units must be light-, medium, or normal-weight conforming to ASTM C90. Allowable loads are based on a safety factor of 5.0.
- 2. Anchors may be installed in any location in the face of the masonry wall (cell, web, bed joint) except within 1-1/4-inch from the of the vertical mortar joint (head joint), center-to-center, provided the minimum edge and end distances are maintained. Anchors may not be placed in the head joint unless the vertical joint is mortared full-depth.
- 3. A maximum of two anchors may be installed in a single masonry cell in accordance with the spacing and edge or end distance requirements. Embedment is measured from the outside surface of the concrete masonry unit to the embedded end of the anchor. See the figure for Illustration of Screw-Bolt+ Anchors Installed into Grouted Concrete Masonry Wall.
- 4. The critical spacing distance, s_{cr}, is the anchor spacing where full load values in the table may be used. The minimum spacing distance, s_{min}, is the minimum anchor spacing for which values are available and installation is permitted. Spacing distance is measured from the centerline to centerline between two anchors.
- 5. The critical edge or end distance, c_{cr}, is the distance where full load values in the table may be used. The minimum edge or end distance, c_{min}, is the minimum distance for which values are available and installation is permitted. Edge or end distance is measured from anchor centerline to the closest unrestrained edge.
- 6. The tabulated values are applicable for anchors installed into the ends of grout-filled concrete masonry units (e.g. wall opening) where minimum edge distances are maintained.
- Load values for anchors installed less than s_{cr} and c_{cr} must be multiplied by the appropriate load reduction factor based on actual spacing (s) or edge distance (c). Load factors are multiplicative; both spacing and edge reduction factors must be considered.
- 8. Linear interpolation of load values between minimum spacing (Smin) and critical spacing (Sco) and between minimum edge or end distance (Cmin) and critical edge or end distance (Cc) is permitted.
- 9. See the figure for Direction of Shear Loading in Relation to Edge and End of Masonry Wall figure for illustration of shear load directions.

Illustration of Screw-Bolt+ Anchors Installed into Grouted Concrete Masonry Wall



Direction of Shear Loading in Relation to Edge and End of Masonry Wall



- 1. Shear load perpendicular to End and parallel to Edge
- 2. Shear load perpendicular to Edge and parallel to End
- 3. Shear load parallel to Edge and perpendicular away from End
- 4. Shear load parallel to End and perpendicular to bottom of wall

Allowable Screw-Bolt+ Tension and Shear Load Capacities Installed into the Tops of Grout-Filled Concrete Masonry Units^{1,2,3,4,5,6,7,8,9,10,11}

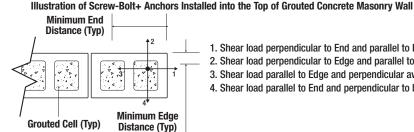




Anchor	Minimum	Minimum	Minimum Edge	Minimum End		Shear Loa	nd, lb (kN)
Diameter d in.	Embedment hnom in. (mm)	Spacing Distance in. (mm)	Minimum Edge Distance in. (mm)	Distance in. (mm)	Tension Load Ibs (kN)	Load Perpendicular to Edge of Masonry Wall (Il to end)	Load Parallel to Edge of Masonry Wall (⊥ to end)
1/4	2-1/2	1-1/2 (38)	1-1/2 (38)	4 (102)	410 (1.8)	185 (0.8)	185 (0.8)
1/4	(64)	1-1/2 (38)	3-1/2 (89)	4 (102)	485 (2.2)	215 (1.0)	215 (1.0)
0./0	2 1/4	2 (51)	1-1/2 (38)	4 (102)	625 (2.8)	225 (1.0)	505 (2.2)
3/8	(83)	2 (51)	3-1/2 (89)	6 (153)	625 (2.8)	560 (2.5)	560 (2.5)
1/2	4-1/4	8 (203)	1-3/4 (45)	8	810 (3.6)	255 (1.1)	580 (2.6)
1/2	(108)	[see Note 4 for reduced minimum spacing distances]	3-3/4 (95)	(203)	1,210 (5.4)	645 (2.9)	1,030 (4.6)
5/8	5 (127)	10 (254)	1-3/4 (45)	10 (254)	900 (4.0)	260 (1.2)	950 (4.2)
3/4	6-1/4 (159)	12 (301)	1-3/4 (45)	12 (305)	1,215 (5.4)	260 (1.2)	990 (4.4)

For SI: 1 inch = 25.4 mm; 1 lbs = 0.0044 kN, 1 psi = 0.006894 MPa.

- 1. All values are for anchors installed in fully grouted concrete masonry wall construction with materials meeting minimum compressive strength, f'm, of 1,500 psi (10.3 MPa). Concrete masonry units must be light-, medium, or normal-weight conforming to ASTM C90. Allowable loads are based on a safety factor of 5.0.
- 2. Anchors may be installed in any location in the top of the masonry wall except within 1-1/4-inch from the of the mortar joint (head joint), provided the minimum edge and end distances
- 3. A maximum of two anchors may be installed in a single masonry cell in accordance with the spacing and edge or end distance requirements. Embedment is measured from the outside surface of the concrete masonry unit to the embedded end of the anchor. See figure for Screw-Bolt+ Anchors Installed into the Top of Grouted Concrete Masonry Wall.
- 4. Minimum spacing distance for 1/2-inch-diameter anchors shall be 8 inches and may be reduced to 2 inches provided the allowable load reduction factor of 0.40 is applied. Linear interpolation may be used to determine the reduction factor for intermediate anchor spacing distances between 8 inches and 2 inches.
- 5. Spacing distance is measured from the centerline to centerline between two anchors.
- 6. Linear interpolation may be used to for 1/4-inch and 3/8-inch-diameter anchors to determine allowable loads for edge distances between 3-1/2-inches and 1-1/2-inches.
- 7. Linear interpolation may be used to for 1/2-inch-diameter anchors to determine allowable loads for edge distances between 3-3/4-inches and 1-3/4-inches.
- 8. The edge and end distance is measured from the anchor centerline to the closest unrestrained edge and end of the CMU block, respectively. See figure for Screw-Bolt+ Anchors Installed into the Top of Grouted Concrete Masonry Wall.
- 9. Spacing distance is measured from the centerline to centerline between two anchors.
- 10. Allowable shear loads parallel and perpendicular to the edge of a masonry wall may be applied in or out of plane, respectively. See figure for Screw-Bolt+ Anchors Installed into the Top of Grouted Concrete Masonry Wall.
- 11. The tabulated allowable load capacities for 1/4-inch-diameter and 3/8-inch-diameter anchors in grout-filled concrete masonry units are not included in the scope of ESR-4042.



- 1. Shear load perpendicular to End and parallel to Edge
- 2. Shear load perpendicular to Edge and parallel to End
- 3. Shear load parallel to Edge and perpendicular away from End
- 4. Shear load parallel to End and perpendicular to bottom of wall

DEWALT.

Allowable Screw-Bolt+ Tension and Shear Load Capacities Installed into the Face of Brick Masonry Walls^{1,2,3,4,5,6,7,8}



				Tension Load				
Anchor	Minimum	Allowable Load		Spacing Distance,	3	E	dge or End Distanc	e
Diameter, d in.	Embedment, hnom in. (mm)	at Car and Sar lbs (kN)	Critical Distance, ser in. (mm)	Minimum Distance, smin in. (mm)	Allowable Load Factor at smin in. (mm)	Critical Distance, cer in. (mm)	Minimum Distance, cmin in. (mm)	Allowable Load Factor at cmin
1/4	1-5/8 (41) 2-1/2	550 (2.4) 830	4 (102)	2 (51)	0.60	3-3/4 (95)	1-1/4 (32)	0.25
3/8	(64) 2 (51)	(3.7) 905 (4.0)	6	3	0.60	6	1-1/2	0.50
	3-1/4 (82) 2-1/2	1,115 (5.0) 1,015	(152)	(76)	0.00	(152)		0.00
1/2	(64) 4-1/4 (108)	(4.5) 1,495 (6.7)	8 (203)	4 (102)	0.60	8 (203)	2-5/8 (68)	0.50
5/8	3-1/4 (83)	1025 (4.6)	10	5	0.50	10	3-3/8	0.50
	5 (127)	2,015 (9.0)	(254)	(127)	0.00	(254)	(86)	0.00
3/4	4 1,815	12	6	0.50	12	4	0.50	
	6-1/4 (159)	2,400 (10.7)	(305)	(152)	0.00	(305)	(102)	0.50

Shear Load

				Spacing Distance,	•	E	dge or End Distanc	e
Anchor Diameter,	Minimum Embedment, hoom	Allowable Load at cor and sor	Critical Distance, sa	Minimum Distance, Smin	Allowable Load	Critical Distance, c _{cr}	Minimum Distance, Cmin	Allowable Load Factor at Cmin
d in.	in. (mm)	lbs (kN)	in. (mm)	in. (mm)	in. (mm)	in. (mm)	in. (mm)	Load Perpendicular to Edge or End
1/4	1-5/8 (41)	405 (1.8)	4	2	0.70	3-3/4	1-1/4	0.20
1/4	2-1/2 (62)	520 (2.3)	(102)	(51)	0.70	(95)	(32)	0.20
3/8	2 (51)	930 (4.1)	6	3	0.70	6	1-1/2	0.20
3/0	3-1/4 (83)	1,030 (4.6)	(152)	(76)	0.70	6 1-1/2 (39)	0.20	
1/2	2-1/2 (64)	1,055 (4.7)	8	4	0.05	8	2-5/8	0.05
1/2	4-1/4 (108)	1,075 (4.8)	(203)	(102)	0.65	(203)	(67)	0.25
F /0	3-1/4 (83)	1,700 (7.6)	10	5	0.50	10	3-3/8	0.40
5/8	5 (127)	1,980 (8.8)	(254)	(127)	0.50	(254)	(86)	0.40
2/4	4 (102)	1,700 (7.6)	12	6	0.50	12	4	0.55
3/4	6-1/4 (159)	2,030 (9.0)	(305)	(152)	0.50	(305)	(102)	0.55

For SI: 1 inch = 25.4 mm; 1 lbs = 0.0044 kN, 1 psi = 0.006894 MPa.

- 1. All values are for anchors installed in minimum two-wythe, solid clay brick masonry walls conforming to ASTM C62, grade SW minimum. Mortar must be type N, S or M. The base material must have a minimum compressive strength, f'm, of 2,000 psi (13.8 MPa). Allowable loads are based on a safety factor of 5.0.
- 2. Anchors may be installed in any location in the face of the masonry wall, provided the minimum edge and end distances are maintained.
- $3. \ \ \text{Embedment is measured from the outside surface of the concrete masonry unit to the embedded end of the anchor.}$
- 4. The critical spacing distance, s_{cr}, is the anchor spacing where full load values in the table may be used. The minimum spacing distance, s_{min}, is the minimum anchor spacing for which values are available and installation is permitted. Spacing distance is measured from the centerline to centerline between two anchors.
- 5. The critical edge or end distance, c_α, is the distance where full load values in the table may be used. The minimum edge or end distance, c_{min}, is the minimum distance for which values are available and installation is permitted. Edge or end distance is measured from anchor centerline to the closest unrestrained edge.
- 6. The tabulated values are applicable for anchors installed into wall openings where minimum edge distances are maintained.
- 7. Load values for anchors installed less than s_{cr} and c_{cr} must be multiplied by the appropriate load reduction factor based on actual spacing (s) or edge distance (c). Load factors are multiplicative; both spacing and edge reduction factors must be considered.
- 8. Linear interpolation of load values between minimum spacing (Smin) and critical spacing (Sw) and between minimum edge or end distance (Cmin) and critical edge or end distance (Cw) is permitted.



STRENGTH DESIGN INFORMATION

Screw-Bolt+ Installation Specifications in Concrete and Supplemental Information 1.2.3.4

or diameter neter of hole kture*	Notation - da	Units -	1/	4										
neter of hole	- da	-				3/8			1/2			5/8		3/4
neter of hole	da		Hex or F	lat Head	Hex	or Flat H	ead	Hex	or Flat H	ead		Hex Head		Hex Head
		in. (mm)	0.2 (6.			0.375 (9.5)			0.500 (12.7)			0.625 (15.9)		0.750 (19.1)
rttai o	d _h	in. (mm)	11/ (8.			1/2 (12.7)			5/8 (15.9)			3/4 (19.1)		7/8 (22.2)
ter (ANSI)	dbit	in.	1/	'4	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$		3/4							
inal epth⁵	h _{nom}	in. (mm)	1-5/8 (41)	2-1/2 (64)		(64)	(83)	(64)	(76)	(108)	(64)	(64)	(127)	4-1/4 (108)
edment	h _{ef}	in. (mm)	1.20 (30)	1.94 (49)										3.08 (78)
depth	h _{hole}	in. (mm)	2 (51)	2-7/8 (73)										4-5/8 (117)
crete ness	h _{min}	in. (mm)	3-1/4 (83)	4 (102)										6 (152)
e distance ⁶	Cmin	in. (mm)			C _{min} for	= 1-1/2 Smin ≥ 3 ((38) 76)							1-3/4 (44)
cing distance ⁶	Smin	in. (mm)	1-1 (3	/2 8)	Si	$_{min} = 2 (5)$	1)							3 (76)
all ,9	lanch	in.	1-3/4	2-5/8	2-1/2	3	4	3	4	5	4	5	6	5
nual que	Tinst,max	ftlbf. (N-m)	19 (26)	25 (34)	25 (34)	25 (34)								70 (95)
act (torque)	Timpact,max	ftlbf (N-m).				300 (407)			300 (407)			700 (950)		700 (950)
rench socket size	-	in.	7/	16		9/16			3/4			15/16		1-1/8
aximum head height	-	in.	21/	64		3/8			31/64			37/64		43/64
ax washer diameter	-	in.							1-1/16			1-1/8		1-13/32
iver size	-	in.										-		-
	-	in.							11/32			-		-
	-	in.							1			-		-
untersunk angle	-	in.	8	2		82			82			-		-
								1						
e stress area body)	Ase	in² (mm²)				0.094 (60.6)			0.176 (113.5)			0.274 (176.8)		0.399 (257.4)
cified ultimate	f _{uta}	ksi (N/mm²)				105 (724)			115 (794)			95 (656)		95 (656)
cified yield strength	f _y	ksi (N/mm²)				84 (579)			92 (635)			76 (524)		76 (524)
ncracked concrete	$eta_{ ext{uncr}}$	lbf/in	_ `	,		1,157,000)		1,014,000)		_ ' /		1,028,000
acked concrete	$eta_{ ext{cr}}$	lbf/in	355,	000		330,000			349,000			378,000		419,000
	pths dment depth rete ess distances	pths nom nom	pths nom (mm) dment hef in. (mm) depth hnole in. (mm) distances hmin in. (mm) distances Cmin in. (mm) ing distances Smin in. (mm) ing distances In. (mm) in. (mm) ing distances Smin ftlbf. (N-m) ing distances In. (mm) in. (mm) ing distances In. (mm) in. (mm) ing distances In. (mm) in. (mm) ing distances In. (mm) in. (mm) in. (mm) ing distances In. (mm) in. (mm) in. (mm) in. (mm) ing distances Timest.max In. (mm) in. (mm) in. (nm) in. (mm) in. (mm) <t< td=""><td>pths nom (mm) (41) dment hef in. (mm) 1.20 (s0) depth hhole in. (mm) 2 (51) rete hmin in. (mm) 3-1/4 (mm) dess hmin in. (mm) 1-7 (mm) distances cmin in. (mm) 1-1 (mm) ing distances smin in. (mm) 1-3/4 ing distances smin in. (mm) (3 ing distances smin in. 1-3/4 1-3/4 ing distances smin ftlbf 19 (N-m) (26) 15 15 (korrell (N-m) (26) 15 inct smin 21/<td>pths nom (mm) (41) (64) dment her in. (mm) 1.20 (30) 1.94 (49) depth hnole in. (mm) 2 2-7/8 (51) 73) rete hmin in. 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(mm) (38) (102) (89) (102) (127) distance⁶ Smin in. 1-1/2 Cmin = 1-1/2 (38) for Smin ≥ 3 (76) Smin = 2 (51) for Smin ≥ 2 (51) smin = 2 (51) for Cmin ≥ 2 (51) smin = 2 (51) for Cmin ≥ 2 (51) smin = 2 (51) for Cmin ≥ 2 (51) smin = 2 (51) for Cmin ≥ 2 (51) smin = 2 (51)</td> <td> Description</td> <td> Description Common Comm</td> <td>the pths</td> <td>pth's pth's /td> <td> Pth Pth</td> <td>the pth</td>	pths nom (mm) (41) dment hef in. (mm) 1.20 (s0) depth hhole in. (mm) 2 (51) rete hmin in. (mm) 3-1/4 (mm) dess hmin in. (mm) 1-7 (mm) distances cmin in. (mm) 1-1 (mm) ing distances smin in. (mm) 1-3/4 ing distances smin in. (mm) (3 ing distances smin in. 1-3/4 1-3/4 ing distances smin ftlbf 19 (N-m) (26) 15 15 (korrell (N-m) (26) 15 inct smin 21/ <td>pths nom (mm) (41) (64) dment her in. (mm) 1.20 (30) 1.94 (49) depth hnole in. (mm) 2 2-7/8 (51) 73) rete hmin in. (mm) 3-1/4 4 (83) (102) distances Cmin in. (mm) (38) (102) distances Smin in. (mm) (38) (38) distances Smin in. 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For SI: 1 inch = 25.4 mm; 1 ksi = 6.894 N/mm²; 1 ft-lb = 1.356 N-m; 1 lb = 0.0044 kN.

- 1. The information presented in this table is to be used in conjunction with the design criteria of ACI 318(-19 or -14) Chapter 17 or ACI 318-11 Appendix D, as applicable.
- 2. For installations in the topside of concrete-filled steel deck assemblies with minimum concrete member thickness, hmin,deck, of 2.5 inches above the upper flute (topping thickness). See the table for anchor setting information for installation on the top of concrete-filled steel deck assemblies and the top of concrete over steel deck installation detail.
- 3. For installations in the topside of concrete-filled steel deck assemblies with sand-lightweight concrete fill, the maximum installation torque, Tinstmax, is 18 ft.-lb.
- 4. For installations through the soffit of steel deck assemblies into concrete, see the design information table for installation in the soffit of concrete-filled steel deck assemblies and the installation details in the soffit of concrete over steel deck for the applicable steel deck profile. Tabulated minimum spacing values are based on anchors installed along the flute with axial spacing equal to the greater of 3h_{ef} or 1.5 times the flute width.
- 5. The embedment depth, hnom, is measured from the outside surface of the concrete member to the embedded end of the anchor.
- 6. Additional combinations for minimum edge distance, cmin, and minimum spacing distance, smin, may be derived by linear interpolation between the given boundary values for the 3/8-inch diameter anchors.
- 7. The listed minimum overall anchor length is based on the anchor sizes commercially available at the time of publication compared with the requirements to achieve the minimum nominal embedment depth. The minimum nominal length for hex head anchors is measured from under the head to the tip of the anchor, the minimum nominal length for flat head anchors is measured from the top of the head to the tip of the anchor.
- 8. The minimum diameter of fixture hole clearance is for the body of the anchor to pass through structural steel members; clearance holes may be 1/8-inch less than tabulated values (same as nominal drill bit diameter) provided the screw anchors are installed through light gauge cold-formed steel members or wood members.
- 9. Hex head anchors with the following minimum lengths are also suitable for use with cold-formed steel members provided the nominal thickness of the fixture attachments does not exceed 20 gauges (0.036-inch base metal thickness):
 - For 3/8-inch-diameters anchors with 2-1/2-inch nominal embedment, 2-1/2-inch long anchors.
 - For 1/2-inch-diameters anchors with 2-1/2-inch nominal embedment, 2-1/2-inch long anchors.
 - For 1/2-inch-diameters anchors with 3-inch nominal embedment, 3-inch long anchors.
 - For 5/8-inch-diameters anchors with 4-inch nominal embedment, 4-inch long anchors.
 - For 5/8-inch-diameters anchors with 5-inch nominal embedment, 5-inch long anchors.
- 10. Mean values shown, actual stiffness varies considerably depending on concrete strength, loading and geometry of application.

Anchor Setting Information for Installation in the Top of Concrete-Filled Steel Deck Assemblies with Minimum Topping Thickness 1,2,3,4

Anahau I	Noneste / Calling Information	Natation	Halta		Nominal Anc	hor Size (inch)	
Anchor F	Property / Setting Information	Notation	Units	1,	/4	3/8	1/2
Head sty	yle	-	-	Hex Head o	or Flat Head	Hex Head or Flat Head	Hex Head or Flat Head
Nominal	anchor diameter	da	in. (mm)		250 .4)	0.375 (9.5)	0.500 (12.7)
	m diameter of hole ce in fixture ⁸	dн	in. (mm)	11, (8	/32 .7)	1/2 (12.7)	5/8 (15.9)
Nominal	drill bit diameter (ANSI)	dbit	in.	1,	/4	3/8	1/2
Minimur	m nominal embedment depth ⁵	h _{nom}	in. (mm)	1-5/8 (41)	2-1/2 (64)	2 (51)	2-1/2 (64)
Effective	embedment	h _{ef}	in. (mm)	1.20 (30)	1.94 (49)	1.33 (33)	1.75 (44)
Minimun	m hole depth	h₀	in. (mm)	2 (51)	2-1/2 (64)	2-3/8 (60)	2-1/2 (64)
	m concrete member thickness thickness)	h _{min,deck}	in. (mm)	2-1/2 (64)	2-1/2 (64)	2-1/2 (64)	2-1/2 (64)
Minimur	m edge distance	Cmin,deck,top	in. (mm)		1/2 88)	2 (51)	2-1/2 (64)
Minimur	m spacing distance	Smin,deck,top	in. (mm)		1/2 88)	2 (51)	2-1/2 (64)
Minimur	n nominal anchor length ^{6,9}	lanch	in.	1-3/4	2-5/8	2-1/2	3
Maximui (torque)	m impact wrench power	T _{impact,max}	ftlb. (N-m)		50 03)	300 (407)	300 (407)
Max. ma	anual installation torque	T _{inst,max}	ftlb. (N-m)	18 ^[7] (26)	25 (34)	25 (34)	45 (61)
ad	Wrench socket size	-	in.	7/	16	9/16	3/4
Hex Head	Max. head height	-	in.	21,	/64	3/8	31/64
완	Max. washer diameter	-	in.	37.	/64	3/4	1-1/16
-	Driver Size	-	in.	T-	30	T-50	T-55
Неас	Max head height	-	in.	13.	/64	21/64	11/32
Flat Head	Max head diameter	-	in.	17.	/32	57/64	1
_	Countersunk angle	-	in.	8	32	82	82

For SI: 1 inch = 25.4 mm; 1 ksi = 6.894 N/mm²; 1 ft-lb = 1.356 N-m; 1 lb = 0.0044 kN.

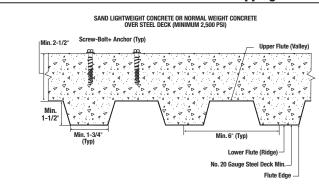
- 1. The anchors may be installed in the topside of concrete-filled steel deck floor and roof assemblies in accordance with this table, the anchor installation specifications in concrete table and the top of concrete over steel deck installation detail provided the concrete thickness above the upper flute meets the minimum thicknesses specified in this table. Minimum concrete member thickness, hmin,deck, refers to the concrete thickness above the upper flute (topping thickness). See the top of concrete over steel deck installation detail.
- 2. Applicable to the following conditions:

ANCHORS & FASTENERS

- For 1/4-inch-diameter anchors with 1-5/8-inch nominal embedment, 2-1/2-inch ≤ hmin,deck < 3-1/4-inch.
- For 1/4-inch-diameter anchors with 2-1/2-inch nominal embedment, 2-1/2-inch $\leq h_{min,deck} < 4$ -inch.
- For 3/8-inch-diameter anchors with 2-inch nominal embedment, 2-1/2-inch ≤ h_{min,deck} < 3-1/2-inch.
- For 1/2-inch-diameter anchors with 2-1/2-inch nominal embedment 2-1/2-inch < hmin deck < 4-1/2-inch
- 3. For all other anchor diameters and embedment depths, refer to the anchor installation specifications in concrete table for applicable values of hmin, cmin and smin, which can be substituted for $h_{\text{min,deck, } cmin, \text{deck,top}} \text{ and } s_{\text{min,deck,top}}, \text{ } \text{respectively}.$
- 4. Design capacities shall be based on calculations according to values in Tension Design Information and the Shear Design Information tables.
- 5. The embedment depth, hnom, is measured from the outside surface of the concrete member to the embedded end of the anchor.
- 6. The listed minimum overall anchor length is based on the anchor sizes commercially available at the time of publication compared with the requirements to achieve the minimum nominal embedment depth, including consideration of a fixture attachment. The minimum nominal length for hex head anchors is measured from under the head to the tip of the anchor, the minimum nominal length for flat head anchors is measured from the top of the head to the tip of the anchor.
- 7. For installations in the topside of concrete-filled steel deck assemblies with normal-weight concrete fill, a maximum installation torque, Tinst,max, of 19 ft.-lb is allowed.
- 8. The minimum diameter of fixture hole clearance is for the body of the anchor to pass through structural steel members; clearance holes may be 1/8-inch less than tabulated values (same as nominal drill bit diameter) provided the screw anchors are installed through light gauge cold-formed steel members or wood members.
- 9. Hex head anchors with the following minimum lengths are also suitable for use with cold-formed steel members provided the nominal thickness of the fixture attachments does not exceed 20 gauges (0.036-inch base metal thickness):
 - For 1/2-inch-diameters anchors with 2-1/2-inch nominal embedment, 2-1/2-inch long anchors.

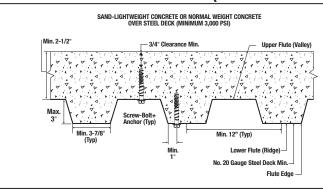


Installation Detail for Anchors in the Top of Concrete Over Steel Deck Floor and Roof Assemblies with Minimum Topping Thickness (See Dimensional Profile Requirements)12



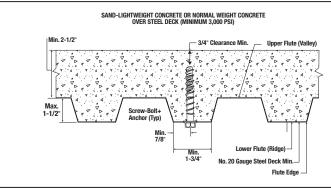
- 1. Anchors may be placed in the top side of concrete over steel deck profiles provided the minimum concrete thickness above the upper flute (topping thickness), minimum spacing distance and minimum edge distances are satisfied as given in Anchor Setting Information for Installation on the Top of Concrete-Filled Steel Deck Assemblies with Minimum Topping Thickness table.
- 2. For all other anchor diameters and embedment depths installed in the top of concrete over steel deck profiles with topping thickness greater than or equal to the minimum concrete member thicknesses given in the Installation Specifications in Concrete table, the minimum spacing distances and minimum edge distances must be used from the Installation Specifications in Concrete table, as applicable.

Screw-Bolt+ Installation Detail for Anchors in the Soffit of Concrete Over Steel Deck Floor and Roof Assemblies (See Dimensional Profile Requirements)^{1,2,3}



- 1. Anchors may be placed in the upper flute or lower flute of concrete-filled steel deck profiles provided the minimum hole clearance of 3/4-inch is satisfied for the selected anchor. See the Tension and Shear Design information for Anchors Installed in the Soffit of Concrete-Filled Steel Deck Assemblies table
- 2. Anchors in the lower flute may be installed with a maximum 15/16 -inch offset in either direction from the center of the flute. The offset distance may be increased proportionally for profiles with lower flute widths greater than those shown provided the minimum lower flute edge distance is also satisfied (e.g. 1-1/4 -inch offset for 4-1/2-inch wide flute).
- 3. See the Tension and Shear Design information for Anchors Installed in the Soffit of Concrete-Filled Steel Deck Assemblies table for design data.

Screw-Bolt+ Installation Detail for Anchors in the Soffit of Concrete Over Steel Deck Floor and Roof Assemblies (See Dimensional Profile Requirements)^{1,2,3}



- 1. Anchors may be placed in the upper flute or lower flute of the concretefilled steel deck profiles provided the minimum hole clearance of 3/4-inch is satisfied for the selected anchor. See the Tension and Shear Design information for Anchors Installed in the Soffit of Concrete-Filled Steel Deck Assemblies table
- 2. Anchors in the lower flute may be installed in the center of the flute. An offset distance may be given proportionally for profiles with flute widths greater than those shown provided the minimum lower flute edge distance
- 3. See the Tension and Shear Design information for Anchors Installed in the Soffit of Concrete-Filled Steel Deck Assemblies table for design data.



Tension Design Information For Screw-Bolt+ Anchor In Concrete^{1,2}



Decision Observatoristic	Natation	Unite					Nor	minal An	chor Dia	meter				
Design Characteristic	Notation	Units	1	/4		3/8			1/2			5/8		3/4
Anchor category	1, 2 or 3	-		1		1			1			1		1
Minimum nominal embedment depth	h _{nom}	in. (mm)	1-5/8 (41)	2-1/2 (64)	2 (51)	2-1/2 (64)	3-1/4 (83)	2-1/2 (64)	3 (76)	4-1/4 (108)	3-1/4 (64)	4 (64)	5 (127)	4-1/4 (108)
Effective embedment	h _{ef}	in. (mm)	1.20 (30)	1.94 (49)	1.33 (34)	1.75 (44)	2.39 (61)	1.75 (44)	2.17 (55)	3.23 (82)	2.24 (57)	2.88 (73)	3.73 (95)	3.08 (78)
\$	teel Strengt	h in Tensio	ı (ACI 31	8-19 17.	6.1, ACI	318-14 1	7.4.1 or	ACI 318	-11 D.5.1	1)				
Steel strength in tension	N _{sa} 10	lb (kN)		535).2)		8,730 (38.8)			20,475 (91.1)			26,260 (116.8)		38,165 (169.8)
Reduction factor for steel strength ^{3,4}	φ	-						C).65					
Concret	e Breakout S	trength in	Tension (ACI 318-	19 17.6.	2, ACI 3	18-14 17	.4.2 or A	ICI 318- 1	1 D.5.2)				
Critical edge distance (uncracked concrete only)	Cac	in. (mm)	4.30 (109)	6.10 (155)	5.00 (127)	6.30 (160)	7.80 (198)	3.30 (84)	5.90 (150)	8.10 (206)	6.30 (160)	7.90 (201)	10.10 (257)	10.90 (277)
Critical edge distance, topside of concrete-filled steel decks with minimum topping thickness ⁹ (uncracked concrete only)	Cac,deck,top	in. (mm)	3.00 (76)	4.00 (102)	3.50 (89)	_11	_11	6.00 (152)	_11	_11	_11	_11	_11	_11
Effectiveness factor for uncracked concrete	Kuncr	-	27	24	30	24	24	30	24	24	30	24	24	27
Effectiveness factor for cracked concrete	k _{cr}	-	1	7		17			17			21		17
Modification factor for cracked and uncracked concrete⁵	$\Psi_{c,N}$	-	1	.0		1.0			1.0			1.0		1.0
Reduction factor for concrete breakout strength ³	φ	-						0.65 (C	ondition	B)				
Pt	ullout Streng	th in Tensio	n (ACI 3	18-19 17	.6.3, AC	318-14	17.4.3 o	r ACI 31	B-11 D.5	.3)				
Characteristic pullout strength, uncracked concrete (2,500 psi) ^{6,10}	N _{p,uncr}	lb (kN)	See N	lote 7	5	See Note	7	5	See Note	7	S	ee Note	7	See Note 7
Characteristic pullout strength, cracked concrete (2,500 psi) ^{6,10}	N _{p,cr}	lb (kN)	765 (3.4)	1,415 (6.3)	5	See Note	7	1,645 (7.3)	2,515 (11.2)	4,700 (20.9)	3,080 (13.7)	4,720 (21.0)	6,900 (30.7)	See Note 7
Reduction factor for pullout strength ³	φ	-						0.65 (C	ondition	B)				
Pullout Strength	in Tension fo	r Seismic <i>A</i>	pplicatio	ns (ACI :	318-19	17.10.3,	ACI 318-	14 17.2.	3.3 or A	CI 318-1	1 D.3.3.3)		
Characteristic pullout strength, seismic (2,500 psi) ^{6,8,10}	Neq	lb	360 (1.6)	1,170 (5.2)	900 (4.0)	1,645 (7.3)	2,765 (12.3)	1,645 (7.3)	2,515 (11.2)		1,910 (8.5)	2,445 (10.9)	3,370 (15.0)	4,085 (18.2)
Reduction factor for pullout strength ³	φ	-						0.65 (C	ondition	B)				

For SI: 1 inch = 25.4 mm; 1 ksi = 6.894 N/mm²; 1 ft-lb = 1.356 N-m; 1 lb = 0.0044 kN.

- 1. The data in this table is intended to be used with the design provisions of ACI 318 (-19 or -14) Chapter 17 or ACI 318-11 Appendix D, as applicable; for anchors resisting seismic load combinations the additional requirements of ACI 318-19 17.10, ACI 318-14 17.2.3 or ACI 318-11 D.3.3, as applicable, shall apply.
- 2. Installation must comply with published instructions and details.
- 3. All values of φ were determined from the load combinations of IBC Section 1605.2, ACI 318 (-19 or -14) Section 5.3, or ACI 318-11 Section 9.2. If the load combinations of ACI 318-11 Appendix C are used, then the appropriate value of φ must be determined in accordance with ACI 318-11 D.4.4. For reinforcement that complies with ACI 318 (-19 or -14) Chapter 17 or ACI 318-11 Appendix D requirements for Condition A, see ACI 318-14 17.3.3(c) or ACI 318-11 Section D.4.3(c), as applicable for the appropriate φ factor when the load combinations of IBC Section 1605.2, ACI 318 (-19 or -14) Section 5.3 or ACI 318-11 Section 9.2 are used.
- 4. The anchors are considered a brittle steel elements as defined by ACI 318 (-19 or -14) 2.3 or ACI 318-11 D.1, as applicable.
- 5. Select the appropriate effectiveness factor for cracked concrete (k_{cr}) or uncracked concrete (k_{uncr}) and use $\Psi_{c,N} = 1.0$.
- 6. For all design cases \(\mathbb{Y}_{cP} = 1.0\). The characteristic pullout strength, \(\mathbb{N}_p\), for concrete compressive strengths greater than 2,500 psi for 1/4-inch-diameter anchors may be increased by multiplying the value in the table by (f'c / 2,500)^{a3} for psi or (f'c / 17.2)^{a3} for MPa. The characteristic pullout strength, \(\mathbb{N}_p\), for concrete compressive strengths greater than 2,500 psi for 3/8-inch- to 3/4-inch-diameter anchors may be increased by multiplying the value in the table by (f'c / 2,500)^{a3} for psi or (f'c / 17.2)^{a3} for MPa.
- 7. Pullout strength does not control design of indicated anchors and does not need to be calculated for indicated anchor size and embedment.
- 8. Reported values for characteristic pullout strength in tension for seismic applications are based on test results per ACI 355.2, Section 9.5.Y
- 9. Anchors are permitted in the topside of concrete-filled steel deck assemblies in accordance with the Installation Detail for Anchors in the Top of Concrete Over Steel Deck Floor and Roof Assemblies with Minimum Topping Thickness.
- 10. Anchors are permitted to be used in lightweight concrete provided the modification factor λ a equal to 0.8 λ is applied to all values of f'c affecting N_n.
- 11. Tabulated critical edge distance values, Cac,deck.top, are for anchors installed in the top of concrete over steel deck profiles with a minimum concrete thickness, hmin, deck, of 2.5 inches above the upper flute (topping thickness). For minimum topping thickness greater than or equal to the minimum concrete member thicknesses, hmin, given in the Installation Specifications table, the associated critical edge distance, Cac, for indicated anchor diameters and embedment depths may be used in the calculation of \(\mathbb{Y}_{cp,N}\) as applicable.



Shear Design Information for Screw-Bolt+ Anchor in Concrete^{1,2,7,8}



Design Characteristic	Notation	Units					Nor	ninal Anc	hor Diam	eter				
Design Gnaracteristic			1.	/4		3/8			1/2			(64) 2.88 (73) 0 12,310 1 (54.8) 1		3/4
Anchor category	1, 2 or 3	-		1		1			1			1		1
Minimum nominal embedment depth	h _{nom}	in. (mm)	1-5/8 (41)	2-1/2 (64)	2 (51)	2-1/2 (64)	3-1/4 (83)	2-1/2 (64)	3 (76)	4-1/4 (108)	3-1/4 (64)		5 (127)	4-1/4 (108)
Effective embedment	h _{ef}	in. (mm)	1.20 (30)	1.94 (49)	1.33 (34)	1.75 (44)	2.39 (61)	1.75 (44)	2.17 (55)	3.23 (82)	2.24 (57)		3.73 (95)	3.08 (78)
	Steel	Strength i	in Shear (ACI 318-	19 17.7.1	ACI 318-	14 17.5.1	or ACI 31	18-11 D.6	i.1)				
Steel strength in shear ⁵	V _{sa}	lb (kN)	1,635 (7.3)	2,040 (9.1)	3,465 (15.4)	3,465 (15.4)	4,345 (19.3)	8,860 (39.4)	8,860 (39.4)	11,175 (49.7)	12,310 (54.8)		15,585 (69.3)	19,260 (85.7)
Reduction factor for steel strength ^{3,4}	φ	-			,			0.6	60					
Steel Stree	ngth in She	ar for Sei	smic App	lications	(ACI 318-	19 17.10	.1, ACI 31	8-14 17.2	2.3.3 or A	CI 318-1 1	D.3.3.3)			
Steel strength in shear, seismic ⁶	V_{eq}	lb (kN)	1,360 (6.1)	1,700 (7.7)	2,415 (10.9)	2,415 (10.9)	3,030 (13.6)	7,090 (31.9)	7,090 (31.9)	8,940 (40.2)	9,845 (44.3)		12,465 (56.1)	15,405 (69.3)
Reduction factor for steel strength in shear for seismic ^{3,4}	φ	-						0.6	60					
C	oncrete Bre	akout Str	ength in	Shear (A0	318-19	17.7.2, A	CI 318-14	17.5.2 oı	r ACI 318-	-11 D.6.2))			
Nominal anchor diameter	da	in. (mm)	0.2 (6	250 .4)		0.375 (9.5)			0.500 (12.7)					0.750 (19.1)
Load bearing length of anchor	le	in. (mm)	1.20 (30)	1.94 (49)	1.33 (34)	1.75 (44)	2.39 (61)	1.75 (44)	2.17 (55)	3.23 (82)	2.24 (57)		3.73 (95)	3.08 (78)
Reduction factor for concrete breakout ³	φ	-						0.70 (Coi	ndition B)					
	Pryout	Strength	in Shear	(ACI 318-	19 17.7.3	3, ACI 318	-14 17.5.	3 or ACI 3	18-11 D.	6.3)				
Coefficient for pryout strength	k _{cp}	-	1	1	1	1	1	1	1	2	1	2	2	2
Reduction factor for pryout strength ³	φ	-						0.70 (Coi	ndition B)					
For St. 1 inch - 25 / mm: 1 kei - 6 90/ N	/mm² 1 ft lh	_ 1 256 N	l m: 1 lh _	0.0044 14	d									

For SI: 1 inch = 25.4 mm; 1 ksi = 6.894 N/mm²; 1 ft-lb = 1.356 N-m; 1 lb = 0.0044 kN.

- 1. The data in this table is intended to be used with the design provisions of ACI 318 (-19 or -14) Chapter 17 or ACI 318-11 Appendix D, as applicable; for anchors resisting seismic load combinations the additional requirements of ACI 318-19 17.10, ACI 318-14 17.2.3 or ACI 318-11 D.3.3, as applicable shall apply.
- 2. Installation must comply with published instructions and details.
- 3. All values of φ were determined from the load combinations of IBC Section 1605.2, ACl 318 (-19 or -14) Section 5.3, or ACl 318-11 Section 9.2. If the load combinations of ACl 318-11 Appendix C are used, the appropriate value of φ must be determined in accordance with ACl 318-11 Section D.4.4. For reinforcement that complies with ACl 318 (-19 or -14) Chapter 17 or ACl 318-11 Appendix D requirements for Condition A, see ACl 318-19 17.5.3, ACl 318-14 17.3.3(c) or ACl 318-11 D.4.3(c), as applicable, for the appropriate φ factor when the load combinations of IBC Section 1605.2, ACl 318 (-19 or -14) Section 5.3, or ACl 318-11 Section 9.2 are used.
- 4. The anchors are considered a brittle steel elements as defined by ACI 318-14 2.3 or ACI 318-11 D.1.
- 5. Reported values for steel strength in shear are based on test results per ACI 355.2, Section 9.4 and must be used for design in lieu of the calculated results using equation 17.7.1.2b of ACI 318-19 or equation 17.5.1.2(b) of ACI 318-14 or equation D-29 in ACI 318-11 D.6.1.2.
- 6. Reported values for steel strength in shear are for seismic applications and based on test results in accordance with ACI 355.2, Section 9.6 and must be used for design.
- 7. Anchors are permitted in the topside of concrete-filled steel deck assemblies in accordance with the Installation Detail for Anchors in the Top of Concrete Over Steel Deck Floor and Roof Assemblies with Minimum Topping Thickness.
- 8. Anchors are permitted to be used in lightweight concrete in provided the modification factor λ a equal to 0.8 λ is applied to all values of f'c affecting N_n.

DEWALT. ANCHORS & FASTENERS

Tension and Shear Design Information for Screw-Bolt+ Anchor in the Soffit (Through the Underside) of Concrete-Filled Steel Deck Assemblies^{1,2,3,4,5,6}



Anakan Buanaka (Calling Information	Natalian	II					Nomina	al Anchor	Diamete	r (inch)				
Anchor Property/Setting Information	Notation	Units	1.	/4		3/8			1/2			5/8		3/4
Minimum nominal embedment depth	h _{nom}	in. (mm)	1-5/8 (41)	2-1/2 (64)	2 (51)	2-1/2 (64)	3-1/4 (83)	2-1/2 (64)	3 (76)	4-1/4 (108)	3-1/4 (64)	4 (64)	5 (127)	4-1/4 (108)
Effective Embedment	h _{ef}	in. (mm)	1.20 (30)	1.94 (49)	1.33 (34)	1.75 (44)	2.39 (61)	1.75 (44)	2.17 (55)	3.23 (82)	2.24 (57)	2.88 (73)	3.73 (95)	3.08 (78)
Minimum hole depth	h₀	in. (mm)	1-3/4 (44)	2-5/8 (67)	2-1/8 (54)	2-5/8 (67)	3-3/8 (86)	2-5/8 (67)	3-1/8 (79)	4-3/8 (111)	3-3/8 (86)	4-1/8 (10.5)	5-1/8 (130)	4-3/8 (111)
Anchors Insta	alled Throug	h the So	ffit of Ste	el Deck A	\ssemblie	s into Co	ncrete (M	linimum 3	3-7/8-inc	h-wide de	eck flute)			
Minimum concrete member thickness ⁷	h _{min,deck,total}	in. (mm)	5-1/2 (140)	5-1/2 (140)	5-1/2 (140)	5-1/2 (140)	5-1/2 (140)	5-1/2 (140)	5-1/2 (140)	5-1/2 (140)	5-1/2 (140)	5-1/2 (140)	6-1/4 (159)	6-1/4 (159)
Characteristic pullout strength, uncracked concrete over steel deck, (3,000 psi)	N _{p,deck,uncr}	lb (kN)	1,430 (6.4)	2,555 (11.4)	2,275 (10.1)	2,655 (11.8)	3,235 (14.4)	2,600 (11.6)	3,555 (15.8)	5,975 (26.6)	2,610 (11.6)	4,150 (18.5)	6,195 (27.6)	6,085 (27.1)
Characteristic pullout strength, cracked concrete over steel deck, (3,000 psi)	N _{p,deck,cr}	lb (kN)	615 (2.7)	1,115 (5.0)	1,290 (5.7)	1,880 (8.4)	2,290 (10.2)	1,230 (5.5)	2,330 (10.4)	4,030 (17.9)	1,600 (7.1)	3,340 (14.9)	4,945 (22.0)	3,835 (17.1)
Characteristic pullout strength, cracked concrete over steel deck,seismic, (3,000 psi)	N _{p,deck,eq}	lb (kN)	290 (1.3)	920 (4.1)	890 (4.0)	1,570 (7.0)	2,015 (9.0)	1,230 (5.5)	2,330 (10.4)	4,030 (17.9)	990 (4.4)	1,730 (7.7)	2,415 (10.7)	3,410 (15.2)
Reduction factor for pullout strength ⁸	φ	-			•			0.0	65					•
Steel strength in shear, concrete over steel deck	V _{sa,deck}	lb (kN)	1,155 (5.1)	2,595 (11.5)	2,470 (11.0)	2,470 (11.0)	3,225 (14.3)	2,435 (10.8)	2,435 (10.8)	5,845 (26.0)	2,650 (11.8)	2,650 (11.8)	6,325 (28.1)	5,175 (23.0)
Steel strength in shear, concrete over steel deck, seismic	Vsa,deck,eq	lb (kN)	960 (4.3)	2,165 (9.6)	1,725 (7.7)	1,900 (8.5)	2,250 (10.0)	1,950 (8.7)	2,095 (9.3)	4,675 (20.8)	2,120 (9.4)	2,325 (10.3)	5,060 (22.5)	4,140 (18.4)
Reduction factor for steel strength in shear for concrete over steel deck ⁸	φ	-						0.	60			,		
Anchors Insta	alled Throug	h the So	ffit of Ste	el Deck A	Ssemblie	s into Co	ncrete (M	linimum 1	1-3/4-inc	h-wide de	eck flute)			
Minimum concrete member thickness ⁷	h _{min,deck,total}	in. (mm)	4 (102)	4 (102)	4 (102)	4 (102)	4 (102)	4 (102)	N.	/A		N/A		N/A
Characteristic pullout strength, uncracked concrete over steel deck, (3,000 psi)	N _{p,deck,uncr}	lb (kN)	1,760 (7.8)	2,075 (9.2)	1,440 (6.4)	2,135 (9.5)	3,190 (14.2)	1,720 (7.7)	N	/A		N/A		N/A
Characteristic pullout strength, cracked concrete over steel deck, (3,000 psi)	N _{p,deck,cr}	lb (kN)	760 (3.4)	910 (4.0)	815 (3.6)	1,510 (6.7)	2,260 (10.1)	1,280 (5.7)	N	/A		N/A		N/A
Characteristic pullout strength, cracked concrete over steel deck,seismic, (3,000 psi)	N _{p,deck,eq}	lb (kN)	355 (1.6)	750 (3.3)	565 (2.5)	1,260 (5.6)	1,985 (8.8)	1,280 (5.7)	N	/A		N/A		N/A
Reduction factor for pullout strength ⁸	φ	-			0.	65			N.	/A		N/A		N/A
Steel strength in shear, concrete over steel deck	Vsa,deck	lb (kN)	1,880 (8.4)	2,315 (10.3)	2,115 (9.4)	2,115 (9.4)	2,820 (12.5)	2,095 (9.3)	N	/A		N/A		N/A
Steel strength in shear, concrete over steel deck, seismic	Vsa,deck,eq	lb (kN)	1,565 (7.0)	1,930 (8.6)	1,475 (6.6)	1,625 (7.2)	1,965 (8.7)	1,675 (7.5)	N	/A		N/A		N/A
Reduction factor for steel strength in shear for concrete over steel deck ⁸	φ	-	0.	60		0.60		0.60	N	/A		N/A		N/A

For SI: 1 inch = 25.4 mm; 1 ksi = 6.894 N/mm²; 1 ft-lb = 1.356 N-m; 1 lb = 0.0044 kN.

- 1. Installation must comply with published instructions and details.
- 2. Values for N_{p,deek,cr} are for sand-lightweight concrete (f'c, min = 3,000 psi) and additional lightweight concrete reduction factors need not be applied. In addition, evaluation for the concrete breakout capacity in accordance with ACI 318-19 17.6.2, ACI 318-14 17.4.2 or ACI 318 D.5.2, as applicable, is not required for anchors installed in the deck soffit (through underside).
- 3. Values for $N_{\text{p,deck,eq}}$ are applicable for seismic loading and must be used in lieu of $N_{\text{p,deck,cr.}}$
- 4. For all design cases $\Psi_{c,P} = 1.0$. The characteristic pullout strength, N_{pn} , for concrete compressive strengths greater than 3,000 psi for 1/4-inch-diameter anchors may be increased by multiplying the value in the table by (f'c / 3,000)^{a3} for psi or (f'c / 17.2)^{a3} for MPa. The characteristic pullout strength, N_{pn} , for concrete compressive strengths greater than 3,000 psi for 3/8-inch- to 3/4-inch-diameter anchors may be increased by multiplying the value in the table by (f'c / 3,000)^{a3} for psi or (f'c / 17.2)^{a3} for MPa.
- 5. Shear loads for anchors installed through steel deck into concrete may be applied in any direction.
- 6. Values of V_{sa,deck.eq} are for sand-lightweight concrete and additional lightweight concrete reduction factors need not be applied. In addition, evaluation for the concrete breakout capacity in accordance with ACI 318-19 17.7.2, ACI 318-14 17.5.2 or ACI 318-11 D.6.2, as applicable, and the pryout capacity in accordance with ACI 318-19 17.7.3, ACI 318-14 17.5.3 or ACI 318-11 D.6.3, as applicable, are not required for anchors installed in the soffit (through underside).
- 7. The minimum concrete member thickness, hmin,deck,total, is the minimum overall thickness of the concrete-filled steel deck (depth and topping thickness).
- All values of φ were determined from the load combinations of 2021 IBC Section 1605.1 or 2018, 2015 and 2012 IBC 1605.2, ACI 318 (-19 or -14) Section 5.3 or ACI 318 Section 9.2. If the load combinations of ACI 318-11 Appendix C are used, then the appropriate value of φ must be determined in accordance with ACI 318-11 D.4.4.



DESIGN STRENGTH TABLES (SD)

Tension and Shear Design Strength Capacities in Cracked Concrete^{1,2,3,4,5,6,7}

					Minim	um Concrete C	compressive St	rength			
Nominal Anchor	Nominal Embed.	f'c = 2,	500 psi	f'c = 3,	,000 psi	f'c = 4,	000 psi	f'c = 6,	000 psi	f'c = 8,0	000 psi
Diameter (in.)	Depth h _{nom} (in.)	ψN₁ Tension (lbs.)	ψV₁ Shear (lbs.)	ψN₁ Tension (lbs.)	φV₁ Shear (lbs.)	φN₁ Tension (lbs.)	φV₁ Shear (lbs.)	ψN₁ Tension (lbs.)	ψV₁ Shear (lbs.)	ψN₁ Tension (lbs.)	ψV₁ Shear (lbs.)
1/4	1-5/8	495	780	525	855	575	980	645	980	705	980
1/4	2-1/2	920	1,225	970	1,225	1,060	1,225	1,195	1,225	1,305	1,225
	2	845	915	930	1,000	1,070	1,155	1,315	1,415	1,515	1,635
3/8	2-1/2	1,280	1,375	1,400	1,510	1,620	1,740	1,980	2,080	2,290	2,080
	3-1/4	2,040	2,200	2,235	2,410	2,580	2,605	3,165	2,605	3,650	2,605
	2-1/2	1,070	1,335	1,170	1,460	1,355	1,690	1,655	2,065	1,915	2,385
1/2	3	1,635	1,900	1,790	2,085	2,070	2,405	2,535	2,945	2,925	3,400
	4-1/4	3,055	5,295	3,345	5,800	3,865	6,695	4,735	6,705	5,465	6,705
	3-1/4	1,850	1,995	2,030	2,185	2,345	2,525	2,870	3,090	3,315	3,570
5/8	4	2,700	5,090	2,960	5,575	3,415	6,435	4,185	7,385	4,830	7,385
	5	3,980	7,400	4,360	8,105	5,035	9,350	6,165	9,350	7,120	9,350
3/4	4-1/4	2,985	6,430	3,270	7,045	3,780	8,135	4,625	9,965	5,340	11,505

Tension and Shear Design Strength Capacities in Uncracked Concrete 1,2,3,4,5,6

		Minimum Concrete Compressive Strength											
Nominal Anchor Diameter (in.)	Nominal Embed. Depth hnom (in.)	f'c = 2,500 psi		f'c = 3,000 psi		f'c = 4,000 psi		f'c = 6,000 psi		f'c = 8,000 psi			
		φNn Tension (lbs.)	φV₁ Shear (lbs.)	φNn Tension (lbs.)	φV _n Shear (lbs.)	φN _n Tension (lbs.)	φV₁ Shear (lbs.)	φNn Tension (lbs.)	φV _n Shear (lbs.)	φN _n Tension (lbs.)	ψV₁ Shear (lbs.)		
1/4	1-5/8	1,155	980	1,265	980	1,460	980	1,785	980	2,065	980		
1/4	2-1/2	2,110	1,225	2,310	1,225	2,665	1,225	2,950	1,225	2,950	1,225		
	2	1,495	1,610	1,640	1,765	1,890	2,035	2,315	2,080	2,675	2,080		
3/8	2-1/2	1,805	1,945	1,980	2,080	2,285	2,080	2,795	2,080	3,230	2,080		
	3-1/4	2,880	2,605	3,155	2,605	3,645	2,605	4,465	2,605	5,155	2,605		
	2-1/2	2,255	1,870	2,475	2,045	2,855	2,365	3,495	2,895	4,040	3,340		
1/2	3	2,495	2,685	2,730	2,940	3,155	3,395	3,865	4,160	4,460	4,805		
	4-1/4	4,530	6,705	4,960	6,705	5,725	6,705	7,015	6,705	8,100	6,705		
	3-1/4	3,270	3,520	3,580	3,855	4,135	4,455	5,065	5,455	5,845	6,295		
5/8	4	3,810	7,125	4,175	7,385	4,820	7,385	5,905	7,385	6,820	7,385		
	5	5,620	9,350	6,155	9,350	7,110	9,350	8,705	9,350	10,050	9,350		
3/4	4-1/4	4,745	10,215	5,195	11,190	6,000	11,555	7,350	11,555	8,485	11,555		
- Anchor Pu	llout/Pryout Strer	ngth Controls 🔲	- Concrete Breal	kout Strength Co	ntrols 🔳 - Steel	Strength Control	s						

- 1- Tabular values are provided for illustration and are applicable for single anchors installed in normal-weight concrete with minimum slab thickness, ha = 1.5hmin, and with the following conditions:
 - c_{a1} is greater than or equal to the critical edge distance, c_{ac} (table values based on $c_{a1} = c_{ac}$).
 - ca2 is greater than or equal to 1.5 times ca1.
- 2- Calculations were performed according to ACI 318-19, Chapter 17. The load level corresponding to the controlling failure mode is listed. (e.g. For tension: steel, concrete breakout and pullout; For shear: steel, concrete breakout and pryout strength in shear are calculated using the effective embedment values, her, for the selected anchors as noted in the design information tables. Please also reference the installation specifications for more information.
- 3- Strength reduction factors (ø) were based on ACI 318-19 Section 5.3 for load combinations. Condition B is assumed.
- Tabular values are determined for static loads only; seismic loading is not considered with these tables.
- 5- For designs that include combined tension and shear, the interaction of tension and shear loads must be calculated in accordance with ACI 318-19, Chapter 17.
- Interpolation is not permitted to be used with the tabular values. For intermediate base material compressive strengths please see ACI 318-19, Chapter 17. For other design conditions including seismic considerations please see ACI 318-19, Chapter 17.
- 7- For seismic design of anchors installed in regions designated as Seismic Design Categories C, D, E or F and in accordance with ACI 318, the tabulated tension design strengths in cracked concrete for concrete breakout and pullout must be multiplied by a factor of 0.75.



Tension and Shear Design Strength Capacities at Minimum Edge Distance, c_{min} for Screw-Bolt+ in Cracked Concrete^{1,2,3,4,5,6,7}



		Minimum Concrete Compressive Strength											
Nominal Anchor	Nominal Embed.	f'c = 2,500 psi		f'c = 3,000 psi		f'c = 4,000 psi		f'c = 6,000 psi		f'c = 8,000 psi			
Diameter (in.)	h _{nom} (in.)	ψN₁ Tension (lbs.)	ψV₅n Shear (lbs.)	ψN₁ Tension (lbs.)	ψV₅n Shear (lbs.)	ψN₁ Tension (lbs.)	ψV₅n Shear (lbs.)	ψN₁ Tension (lbs.)	φV sn Shear (lbs.)	ψN₁ Tension (lbs.)	ψVsn Shear (lbs.)		
1/4	1-5/8	495	310	525	335	575	390	645	475	705	550		
1/4	2-1/2	920	340	970	370	1,060	430	1,195	525	1,305	605		
	2	685	355	755	390	870	450	1,065	550	1,230	635		
3/8	2-1/2	875	375	960	410	1,110	475	1,355	580	1,565	670		
	3-1/4	1,195	400	1,310	435	1,510	505	1,850	620	2,140	715		
	2-1/2	960	515	1,050	565	1,215	650	1,485	800	1,715	920		
1/2	3	1,170	540	1,280	590	1,480	680	1,810	835	2,090	960		
	4-1/4	1,765	580	1,935	640	2,230	735	2,735	900	3,155	1,040		
	3-1/4	1,205	580	1,320	635	1,525	730	1,870	895	2,155	1,035		
5/8	4	1,560	610	1,705	665	1,970	770	2,415	945	2,790	1,090		
	5	2,075	640	2,270	700	2,625	810	3,215	995	3,710	1,145		
3/4	4-1/4	1,675	650	1,835	715	2,120	825	2,595	1,010	2,995	1,165		

Tension and Shear Design Strength Capacities at Minimum Edge Distance, Cmin for Screw-Bolt+ in Uncracked Concrete 12.34.56

Nominal Anchor			Minimum Concrete Compressive Strength												
	Nominal Embed.	f'c = 2,	500 psi	f'c = 3,000 psi		f'c = 4,000 psi		f'c = 6,000 psi		f'c = 8,000 psi					
Diameter (in.)	h _{nom} (in.)	ψN₁ Tension (lbs.)	ΦV₅n Shear (lbs.)	ψN₁ Tension (lbs.)	φV₅n Shear (lbs.)	ØN₁ Tension (lbs.)	φν _{sn} Shear (lbs.)	ψN₁ Tension (lbs.)	ΦV₅n Shear (lbs.)	ψN₁ Tension (lbs.)	ψVsn Shear (lbs.)				
1/4	1-5/8	420	430	460	470	530	545	650	670	750	770				
1/4	2-1/2	650	475	715	520	825	600	1,010	735	1,165	850				
3/8	2	485	495	530	545	610	630	750	770	865	890				
	2-1/2	515	525	565	575	650	665	800	815	920	940				
	3-1/4	775	560	850	610	980	705	1,200	865	1,385	1,000				
	2-1/2	1,345	720	1,475	790	1,705	910	2,085	1,120	2,410	1,290				
1/2	3	910	755	1,000	825	1,150	955	1,410	1,165	1,630	1,345				
	4-1/4	1,490	815	1,630	895	1,885	1,030	2,310	1,265	2,665	1,460				
	3-1/4	1,135	810	1,245	890	1,435	1,025	1,760	1,255	2,030	1,450				
5/8	4	1,205	850	1,320	935	1,520	1,080	1,865	1,320	2,150	1,525				
	5	1,620	895	1,775	985	2,050	1,135	2,515	1,390	2,900	1,605				
3/4	4-1/4	1,130	910	1,235	1,000	1,425	1,155	1,745	1,415	2,015	1,630				

- 1- Tabular values are provided for illustration and are applicable for single anchors installed in normal-weight concrete with minimum slab thickness, $h_a = h_{min}$, and with the following conditions:
 - c_{a1} is greater than or equal to the minimum edge distance, c_{min} (table values based on $c_{a1} = c_{min}$).
 - ca2 is greater than or equal to 1.5 times ca1.
- 2- Calculations were performed according to ACI 318-19, Chapter 17. The load level corresponding to the controlling failure mode is listed. (e.g. For tension: steel, concrete breakout and pullout; For shear: steel, concrete breakout and pryout). Furthermore, the capacities for concrete breakout strength in tension and pryout strength in shear are calculated using the effective embedment values, her, for the selected anchors as noted in the design information tables. Please also reference the installation specifications for more information.
- 3- Strength reduction factors (ø) were based on ACI 318-19 Section 5.3 for load combinations. Condition B is assumed.
- 4- Tabular values are determined for static loads only; seismic loading is not considered with these tables.
- 5- For designs that include combined tension and shear, the interaction of tension and shear loads must be calculated in accordance with ACI 318-19, Chapter 17.
- 6- Interpolation is not permitted to be used with the tabular values. For intermediate base material compressive strengths please see ACI 318-19, Chapter 17. For other design conditions including seismic considerations please see ACI 318-19, Chapter 17.
- 7- For seismic design of anchors installed in regions designated as Seismic Design Categories C, D, E or F and in accordance with ACl 318, the tabulated tension design strengths in cracked concrete for concrete breakout and pullout must be multiplied by a factor of 0.75.



ORDERING INFORMATION





Screw-Bolt+

							20V Max*	SDS Plus Rotary	/ Hammers	Flexvolt SDS Max
	Cat. No.		Anchor Size	Approximate Thread Length	Box Qty.	Ctn. Qty.	DCH273P2DH 1" L-Shape	DCH133M2 1" D-Handle	DCH293R2 1-1/8" L-Shape w/ E-Clutch	DCH481X2 1-9/16" w/ E-Clutch
Hex	Head	Flat Head]					Carbio	la Rite	
Zinc Plated	Galvanized	Zinc Plated						Odibit	ic bits	
PFM1411000	-	-	1/4" x 1-1/4"	1-1/4"	100	600	DW5517	DW5417	DW5417	-
PFM1411020	-	-	1/4" x 1-3/4"	1-5/8"	100	600	DW5517	DW5417	DW5417	-
PFM1411060	-	-	1/4" x 2-1/4"	1-5/8"	100	600	DW5517	DW5417	DW5417	-
PFM1411080	-	-	1/4" x 2-5/8"	2-1/2"	100	500	DW5517	DW5417	DW5417	-
PFM1411100	-	PFM1411105	1/4" x 3"	2-1/2"	100	500	DW5517	DW5417	DW5417	-
PFM1411160	-	-	3/8" x 1-3/4"	1-3/4"	50	300	DW5527	DW5427	DW5427	-
PFM1411220	-	PFM1411225	3/8" x 2-1/2"	2"	50	300	DW5527	DW5427	DW5427	-
PFM1411240	PFM1461240	PFM1411245	3/8" x 3"	2"	50	250	DW5527	DW5427	DW5427	-
PFM1411280	PFM1461280	PFM1411285	3/8" x 4"	3-1/4"	50	250	DW5527	DW5427	DW5427	-
PFM1411300	PFM1461300	-	3/8" x 5"	3-1/4"	50	250	DW5529	DW5429	DW5429	-
PFM1411320	PFM1461320	-	3/8" x 6"	3-1/4"	50	150	DW5529	DW5429	DW5429	-
PFM1411340	-	-	1/2" x 2"	1-3/4"	50	200	DW5537	DW5437	DW5437	-
PFM1411360*	-	-	1/2" x 2-1/2"	2-1/2"	50	200	DW5537	DW5437	DW5437	-
PFM1411380	-	PFM1411385	1/2" x 3"	2-1/2"	50	150	DW5537	DW5437	DW5437	-
PFM1411420	PFM1461420	PFM1411425	1/2" x 4"	2-1/2"	50	150	DW5537	DW5437	DW5437	-
PFM1411460	PFM1461460	PFM1411465	1/2" x 5"	4-1/4"	25	100	DW5538	DW5438	DW5438	-
PFM1411480	PFM1461480	-	1/2" x 6"	4-1/4"	25	75	DW5538	DW5438	DW5438	-
PFM1411520	PFM1461520	-	1/2" x 8"	4-1/4"	25	100	DW5538	DW5438	DW5438	-
PFM1411540	-	-	5/8" x 3"	2-3/4"	25	100	DW5471	DW5446	DW5471	DW5806
PFM1411580	-	-	5/8" x 4"	3-1/4"	25	100	DW5471	DW5446	DW5471	DW5806
PFM1411600	PFM1461600	-	5/8" x 5"	5"	25	75	DW5471	DW5446	DW5471	DW5806
PFM1411640	PFM1461640	-	5/8" x 6"	5"	25	75	DW5471	DW5446	DW5471	DW5806
PFM1411680	PFM1461680	-	5/8" x 8"	5"	25	50	DW5471	DW5447	DW5471	DW5806
PFM1411700	-	-	3/4" x 3"	3"	20	60	DW5474	DW5453	DW5474	DW5810
PFM1411720	-	-	3/4" x 4"	3"	20	60	DW5474	DW5453	DW5474	DW5810
PFM1411760	-	-	3/4" x 5"	4-1/4"	20	60	DW5474	DW5453	DW5474	DW5810
PFM1411800	PFM1461800	-	3/4" x 6"	4-1/4"	20	60	DW5474	DW5453	DW5474	DW5810
PFM1411840	PFM1461850	-	3/4" x 8"	4-1/4"	10	40	DW5474	DW5455	DW5474	DW5810
PFM1411880	-	-	3/4" x 10"	4-1/4"	10	20	DW5475	DW5455	DW5475	DW5812
Anchors not long of ICC-ES ESR-3 Catalog numbers v	Shaded catalog numbers denote sizes which are less than the minimum standard anchor length for Strength Design. Anchors not long enough to meet the minimum nominal embedments published for strength design are outside the scope of ICC-ES ESR-3889. Catalog numbers with an asterisk (*) denote sizes that meet the minimum anchor length requirement for strength design provided the fixture attachment does not exceed 0.036-inch (0.91 mm) in thickness							um Tool Match num Tool Match ecommended		

provided the fixture attachment does not exceed 0.036-inch (0.91mm) in thickness.

The selected anchor length should be long enough to accommodate the attachment thickness and achieve the minimum nominal embedment into the base material required for the application.

The published size includes the nominal diameter and length of the anchor. The length is measured from under the head for hex head parts and from the top of the head for flat head (countersunk) parts.

TECHNICAL GUIDE - MECHANICAL ANCHORS @2024 DEWALT - REV. H

Impact Wrench Selection Guide

ANCHORS & FASTENERS

Anches Cotting Information	Nominal Anchor Diameter (Inch)										
Anchor Setting Information	1/4"		3/8"		1/2"		5/8"		3/4"		
Max Impact Wrench Power	150 ft-lbs		300 ft-lbs		300 ft-lbs		700 ft-lbs		700 ft-lbs		
	FULL	SPEED 1	SPEED 1	SPEED 2	SPEED 1	SPEED 2	SPEED 2	SPEED 3	SPEED 2	SPEED 3	
Suggested 20V Max Impact Wrench, Tool Setting / Speed and Cat. No.	DCF902	DCF921, DCF922, DCF923, DCF891, DCF892, DCF900	DCF911, DCF913, DCF900	DCF921, DCF922, DCF923, DCF891, DCF892	DCF911, DCF913, DCF900	DCF921, DCF922, DCF923, DCF891, DCF892	DCF900	DCF891, DCF892	DCF900	DCF891, DCF892	

DEWALT Impact Wrenches



















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Cat. No.	DCF901	DCF903	DCF911	DCF913	DCF921	DCF922	DCF923	DCF891	DCF892
Anvil Size	3/8"	1/2"	3/8"	1/2"	1/2"	3/8"	1/2"	1/2"	1/2"
Anvil Type	Hog Ring	Detent	Hog Ring	Hog Ring	Detent				
MAX Fastening Torque	Speed 1: 250 ft-lbs	Speed 1: 250 ft-lbs	Speed 1: 250 ft-lbs	Speed 1: 250 ft-lbs	Speed 1: 100 ft-lbs Speed 2: 300 ft-lbs	Speed 1: 100 ft-lbs Speed 2: 300 ft-lbs	Speed 1: 100 ft-lbs Speed 2: 300 ft-lbs	Speed 1: 100 ft-lbs. Speed 2: 300 ft-lbs. Speed 3: 600 ft-lbs	Speed 1: 100 ft-lbs. Speed 2: 300 ft-lbs. Speed 3: 600 ft-lbs